

PHYSIOLOGICAL MODEL OF THE RESPONSE OF FLOATING
ACCUMULATED REFORMABLE MEMBRANES

By

ROBERT J. PETTELL

A DISSERTATION PRESENTED TO THE GRADUATE SCHOOL
OF THE UNIVERSITY OF FLORIDA IN
PARTIAL FULFILLMENT OF THE REQUIREMENTS
FOR THE DEGREE OF DOCTOR OF PHILOSOPHY

UNIVERSITY OF FLORIDA

1988

U OF F LIBRARY

ACKNOWLEDGMENTS

The author is indebted to Dr. Larry B. Siegfelt, her major professor, for his support, encouragement, advice and assistance throughout the course of this work.

The author is also grateful to Dr. Glen B. Giergiele, Dr. Del Bettscher, Dr. Cheng T. Hsu, and Dr. Wayne Wilson for their advice and time. Appreciation is extended to the Civil Engineering Department for the use of the hydraulics laboratory and to B. A. Christensen and E. DeLeon for their help there.

The author is very grateful to Mike Simpson, Jorge Rios, Rosalind Kline, and her sister Janet Pettit for their sense of adventure, courage and strength, and for their help with the water hyacinth experiments.

Finally, the author appreciates deeply the constant support and encouragement from her husband Ricardo Rodriguez, and the presence of her family and friends.

TABLE OF CONTENTS

ACKNOWLEDGMENTS	vi
LET TO READER	vii
ABSTRACT	viii
CHAPTERS	
1. INTRODUCTION	1
2. OBJECTIVE	2
3. PHYSICAL CHARACTERISTICS OF WATER-SOLUBLE POLYMERS	4
Review of Literature	6
Morphological Description of Water-Soluble Polymers	6
Chemical Composition	11
Density and Viscosity	12
Molecular Weight and Molecular Weight Distribution	13
Sample Size and Plant Characteristics	13
Plant Area Distribution and Moisture Content	14
Connectivity, Geometry, and Area Distribution	14
Density and Viscosity	15
Plant Area Distribution Histograms	15
Moisture Content	17
Area Density	17
Connectivity and Geometry	18
Summary	19
4. KINEMATICS AND ELASTIC PROPERTIES	20
Kinematics and Elastic Properties	20
Review of Literature	21
Elastic Model for Water-Soluble Polymers	21
Materials and Methods	22
Plant material	22
Testing equipment	23
Analysis of data	24

Results and Discussion	56
Experimental observations	56
Results of regression analysis	57
Drug coefficient	58
Summary	74
Static Characteristics	74
Long-term Service	74
Materials and Methods	76
Results and Discussion	80
Summary	87
5. MATHEMATICAL MODEL OF WATER SWELLING TIME	93
Conceptual Model	93
Element Description	94
Model Formulation	95
Method of Solution and Parameter Estimation	95
Results of Parameter Estimation and Model Analysis	99
Summary	111
6. SATURATION MODEL	113
Conceptual Model	113
Model Formulation	115
Method of Solution and Parameter Estimation	118
Experimental Compression Tests	119
Results and Discussion	124
Summary	126
7. CONCLUSIONS AND RECOMMENDATIONS FOR FURTHER RESEARCH	127
APPENDICES	
A. Friction Solvent Program	140
B. Friction Solvent Program for Compression Characteristics	146
REFERENCES	151
BIBLIOGRAPHIC NOTES	155

REF TO SYMBOLS

English Symbols

α	characteristic length
d	semi-circular width
δ	area
A_p	projected flat area of a ribnose = $R_p \cdot l_p$
A_n	specific net area = A_p / R_p
A_n	net area as a function of time
b	roughness element width
k_L	laminar plate friction coefficient
C	Chen discharge coefficient, 1/s $R^{1/2}$
C_D	drag coefficient
C_n	experimental proportionality constant 1000 $R_p A_p C_n$
C_1, C_2	experimental constants for flow in tapered channel
d	deviation of actual value from computed value
E_n	maximum expected net compression value, μ
h_p	drag force
R_p	ribnose diameter

D_p	variable used to determine steady drag coefficient
e	dimension spacing length of roughness element
E	Young's elastic modulus
F	friction factor for smooth flow over a bank of spindles
F_a	sliding friction contact force
F_b	elastic force
F_g	inertia
g	acceleration due to gravity
Q	mean flow rate
h	roughness element height
I	moment of inertia
h_g	height of grade for equivalent sand roughness
K	elastic constant
L	flow length
L	smaller base length
L_p	obstacle length
L	initial gap length
L'	variable used to determine steady drag coefficient
n	Reynolds number exponent
N	mean
N_p	total mass of plants in a unit
n	Manning's roughness coefficient
N	statistical sample size
N_p	number of plants in a unit
P	number of major contributions encountered by flow through a bank of spindles

P	applied load
q	statistical method used for determination of spurious errors
r	cylinder radius
R	hydrostatic radius
σ_a	standard error of coefficient
σ_b	standard error of constant
τ	deformation of diametrical mid section of a cylinder
V	velocity of body relative to undisturbed fluid
V_{cr}	critical velocity $= \sqrt{v_\infty^2/g}$
x	normal approach of a flat plate to the center of a cylinder
Y	net yield
z	longitudinal spacing of roughness haffle
Z	axial displacement
Z	half-angle of contact between a cylinder and a flat plate
Z_0	normal depth of flow

Greek Symbols

δ	deflections or deformation
ϵ	kinematic viscosity
ρ	density
τ_w	shearing stress at a wall
ν	Stokes's ratio
ω_n	natural frequency

Abstract of Dissertation Presented to the Graduate School
of the University of Florida in Partial Fulfillment of the
Requirements for the Degree of Doctor of Philosophy

**HYDROMECHANICAL MODEL OF THE MOVEMENT OF FLOATING
AGGREGATED SUBMERGED WEEDS**

By

Stephen J. Petraitis

December 1986

Chairman: Larry E. Regehr and J. Wayne Hight
Major Department: Agricultural Engineering

In this study, simplified machine plant interactions were analyzed so that more efficient harvesters could be designed. Plants investigated were water hyacinths (*Eichhornia crassipes*), which are floating hydrophytes that grow together in deep mats or aggregates. Because mechanical harvesting is expensive and time consuming, they are currently harvested to control their propagation in water bodies only where a chemical or biological method is prohibited or unsuccessful. More efficient harvesting methods will depend on equipment designs based on hydro-mechanical characterization of mats, which up to now have been inadequately known in predict performance of testing or removal systems.

Conceptual models were developed that describe mooring water hydraulic interactions for two different harvesting operations: rectangular net towing and forced compression. The mathematical description of nets was based on experimental observations and physical and hydro-mechanical principles. Primary system variables are horizontal displacement and velocity of the net, and the input variable is either towing or compression velocity as a function of time.

Equations derived from the conceptual models were solved numerically, and output variables such as compressing and towing forces, net instability, and change in net length, were predicted over time with good correlation with experimental data. Drag coefficients are found to be independent of Reynolds number but dependent on net geometry. Power input to harvesters can be reduced by careful selection of acceleration patterns, towing or compression velocity, net geometry, and mooring point distribution.

CHAPTER 1

INTRODUCTION

Water hyacinths (*Eichhornia crassipes*) are free-floating, emergent hydrophytes or, in mechanical terms, deformable bodies that are positively buoyant. These plants grow together to form a mat or aggregate and they are harvested to control their propagation in natural waterways, canals and some sewage treatment systems. In Florida, water hyacinths were sighted in swamps of old aquatic plants surveyed in 1886, and they are present because they cling to waterways (Roberts, 1989). Florida's Surface Water Enhancement and Management Act (1987) includes aquatic plant management strategies to restore water bodies. Additional restrictions on pesticide use by the Environmental Protection Agency and the U. S. Fish and Wildlife Service will now apply to herbicides used within ranges of protected plants and animals (Hobby, 2007). These political issues and increased use of water hyacinths in water treatment systems across the impetus of the development of efficient and low ways to manage aquatic plants without the use of heavy, expensive mechanical equipment or potentially dangerous toxins and without the time delay often found in biological control methods.

The current cost to mechanically harvest water hyacinths is about 10 to 150 dollars per wet ton (Bogull et al., 1987). Typically, 200

tons of plants are found in one hectare; consequently, the most economical one hectare is \$500 to \$1000. On the other hand, chemical treatment costs approximately \$1000/ha. The U.S. Army Corps of Engineers considers a system which harvests and disperses 80 to 100 tons per hect. to be efficient enough to control insect growth rates of the water hyacinth (Osajepet and Smith, 1976). The Corps also found that the most advanced harvesting system available can not meet these conditions, and that the principal problem is the transportation of the plants over the water. For instance, one way of transportation is using a net of plants. Often, however, when a net is used, the plants tend to "jet". Report (1976) described the mechanism of jet formation as a combination of attraction of plants and a rolling under which they harvest the vertical dimension of the net. As a consequence of a jet, towing force increases, and eventually a net pulls under the containment device and escapes.

An efficient hyacinth harvester, then, depends on simple equipment that is based on hydro-mechanical characteristics-- (e.g. drag forces and the elastic properties) of water hyacinth roots--however, physical characterization of aquatic plants has been inadequate to predict the performance of large towing or removal systems. The purpose of this research was to determine hydro-mechanical properties of water hyacinths necessary to predict the response of a water hyacinth net to various forces as velocity inputs and boundary restrictions.

CHAPTER II OBJECTIVES

The principal objectives of this research are listed below, and each objective will be the focus of a subsequent chapter.

- Quantitatively describe those physical characteristics of water hyacinths that distinguish one type of aggregation from another.

- Determine the hydrodynamic and elastic constant properties of water hyacinths, and find the equations that describe the relationships between viscous forces on mats and towing velocity and between surface forces and plant displacement within the mat.

- Develop, from developed constitutive relations and structural equations, a mathematical model that predicts how a particular aggregation of water hyacinths reacts to an applied velocity.

- Validate the model and demonstrate practical applications for

CHAPTER III
PHYSICAL CHARACTERIZATION OF WATER BEACONS
(Molecular form)

Several physical characteristics of individual water lyophilic plants and of different aggregations are described in this chapter: standing density, plant mass and structure, unit frequency and connectivity. The latter is a new property. Generally, these characterizations vary according to habitat, season and plant age, and often between species.

First, the morpho-anatomical description of water lyophilic provides expected ranges of plant mass, leaf, root, and rhizome length and diameter, and the internal cellular structure of the plants. This information is basic for the determination of leaves (e.g. plant mass) and for the identification of the plant parts that physically interact with water or with other plants.

The shape of rhizomes and roots, the principal plant parts projecting from the water, influences the viscous forces generated when the plants are moved. An empirical method to predict rhizome lengths from other physical descriptions would be useful. Rhizomes are difficult to measure, because they are usually heavily covered with roots and debris.

Standing biomass density, i.e. combination with increasing speed and width, decreases the capacity of a harvester and the power requirements for conveying or transportation systems. Also, plant mass affects inertial forces encountered in a loading operation. On the other hand, areal population density is useful in flow models because total projected area in flow depends on the number of plants. The measurement of areal population density in flow conveying systems is requires handling every plant, yet, an empirical method to produce population density from other physical descriptions (e.g. plant mass or length) does not exist.

Bed frequency, the force required to submerge a given area of water hyacinths, is useful for the design of conveyor, boom and containment structures; since the plants under certain conditions will under such designs. Water hyacinth plants can be connected through leaf entanglements and roots. Beds have been characterized by either standing population or biomass density and, to a limited extent, by frequency, but the literature reports no property that could quantify the "connectiveness" of a bed. Therefore, a test described in this chapter was developed to measure bed connectiveness. Information on connectiveness is needed when a mat is to be separated into smaller units for feeding or lifting from the water. Also, information on connectiveness changes could be useful in biological studies. Rapid decreases in connectiveness to a mat could indicate that the plants have been subjected to physiological stresses.

The objectives of this chapter are:

-draw from available literature the nomenclature and composition of water hyacinths

- Determine the distribution of mass in a water hyacinth aggregation and how that distribution could vary from one population to another
- Develop a relation between standing density and average plant size of a mat
- Develop an empirical method to calculate rhizome length
- Determine how one biophysical metric for different mats, and find a relation with the use of plant size characteristics to predict it
- Determine seasonality for different mat types

Review of Literature.

Macro-environmental Description of the Water Hyacinth.

The mature water hyacinth plant is a free-floating (sessile), emergent hydrophyte that consists of roots, rhizomes, stolon, leaves, inflorescences and fruit clusters (Figure 3-0). In standing water conditions, stolon to root length ratio in the range of 1-2 is common and 80% of the total plant dry matter is in the leaves and petioles. In nutrient-rich conditions, stolon to root ratios are in the range of 4-6, and up to 80% of the dry matter is in the foliage (Boddy, 1988). Medium-sized water hyacinths (500-2000 gwt dw) are harvested from average treatment plants because plant densities are optimum for reducing nutrient biomass yields (Boddy, 1988).

The fibrous root (as opposed to a tap root) has a dense cluster of main and lateral roots. The lateral roots are not randomly dispersed, and they are arranged in groups along the main root mass (Bullock et al., 1992). The roots of plants from different habitats and seasons vary little in diameter but can vary greatly in length. The diameter varies from 0.4 to 1.0 mm. The length varies from 4 to

Figure 3.13 is a vector diagram showing the forces of interest, costs, a premium, and profits.



15 cm in small plants; 40 to 50 cm in medium plants, and 10 to 20 cm in large plants (Preston and Burke; 1961 and Zanner et al., 1982).

A stem has distinct regions: epidermis, cortex and stela. The epidermal cells are either oblong, square or rectangular in shape and are 20 to 40 μ m in size. The cortical regions consist of an outer layer of parenchyma cells, a cortical lacuna that occupies 75% of the cortex, an inner layer of parenchyma cells, and an endodermis that occupies 50% of the cortex. The stela consists of a xylem region, 10 to 15 layers of parenchyma cells, and 10 to 15 cells of phloem (Zanner et al., 1982).

A rhizome is a stem or axis with short internodes. The rhizome produces at the numerous nodes all the roots, leaves, inflorescences, and inflorescences of the plant. The diameter of the stem varies from 1 to 3 cm and the length from 1 to 30 cm. Structurally similar to a root, a rhizome consists of epidermis, cortical and stela regions. The stela contains very few fibers, but some tracheary elements are present which increase in number and size as the rhizome matures. The cortical region consists of parenchyma tissue and air spaces (Zanner et al., 1982).

A leaf consists of a petiole, a float (in small plants or those living in very open areas) a blade and possibly an internode (distance portion between the float and the blade). Floats are located in the petiole. They are round or pear-shaped and range from 3 to 4 cm in diameter. They are formed when the physiological availability of water to the plant is great (Jain, 1959); hence, in very dense stands of water hyacinths, floats are usually absent. Petiole length increases with increasing plant crowding density (Tucker, 1981). At 5

leaf's $\frac{1}{2}$ (ab); petiole length was 20 cm. vs 20 kg kg^{-2} (ab); length was 40 and vs 40 kg kg^{-2} (ab); length was 50 cm. Generally, the first type of leaves are disposed in a nearly horizontal position (45 to 60° from horizontal) whereas the opposite leaves (e.g., leaves whose bases overlap within or above them) approach verticality (75° from the horizontal) (Freeman and Marks, 1964). In dense stands, water hyacinths' leaves grow upright rather than horizontally over the water surface (Shetty, 1966).

Stems originate from a rhizome and produce new offshoots at apicalities at their distal ends. Stems can be relatively short and vertical in dense stands but long and horizontal in open conditions. They are 1 to 1.5 cm in diameter and 4 to 20 cm long in open stands or 5 to 6 cm long in dense stands (Freeman and Marks, 1964 and Zenger et al., 1982).

Internal structures of various organs of the aerial part of a plant are distinct to each other (Jensen et al., 1982) and they can be classified into three groups: parenchymatous cells, support and conductive cells, and air spaces. Nearly-five percent of the biomass of a plant is parenchymatous or consists of unspecialized cells that usually have well-defined central vacuoles and they are used, typically for storage of proteins, starch grains, oil, etc., as an oligosaccharide cells in the rhizome.

Collateral vascular bundles, a collection of phloem and xylem cells, are scattered throughout the aerial part of a plant. Phloem functions in conduction of food and xylem functions in conduction of water and minerals and in support. Besides vascular bundles, fiber and tracheids, which are types of xylem providing mechanical support,

are usually found in a plant. Interwoven fibers are abundant in blades, roots and petioles, but rare in culms and rhizomes. While tracheids can be located throughout an entire plant. The fibers are 1.5 to 3 m long by 2 to 10 μ m in diameter, whereas tracheids are elongated and helical in shape and can be stretched far before they break.

Air spaces allowing water hyssopides to float are located in abundant intercellular spaces: air chambers and parenchyma cells. Air chambers or lacunae are particularly characteristic to parenchyma tissue and they appear as narrow passages where cells seem to have been pulled apart at the corners by the continuous growth of the epidermis. The air chambers permit passive diffusion of gases and dissolved proteins. The cross patterns of the air chambers, known as diaphragms, prevent flooding because these structures are perforated with minute openings specific only for the passage of gases. The system of lacunae is continuous from leaves to roots.

General Composition

Basically, the water hyssopach consists of water. Redmond and Eadie (1966) and Snijling et al. (1971) determined that 50 to 60% of the fresh content of roots, 55-18% of rhizomes, 55-75% of culms, 55-65% of stems, and 67 to 80-92% of blades is water.

Depending on the standing density of plants, 50 to 60% of the dry weight of a plant is fiber (cellulose-hemicellulose and lignin), around 40% is carbon, and 1-2 to 25% is nitrogen, (Delouis and Snijling, 1984)

Density and Biomass

The densities of various parts of plants were determined by the volume weights method by Fiedler and Burke (1988). We found that the specific weight of a root was 0.362, of a rhizome, 0.803, of a stem, 0.818, of a flower, 0.156 and of a blade, 0.741.

Fiedler and Burke (1988) also observed the loading capacity defined as the weight required to submerge a mat of plants, of a 1.5 m² mat of plants to be 177 to 428 kgm² (t/m). The latter corresponded to a population density of 200 plantsm².

Statistics and Methods

Extensive literature review concerning growth rates, morphology and chemical composition of water hyacinths. Insufficient information existed to establish criteria that could be used to physically distinguish different types of mats. This section presents the procedures that were performed to determine characteristics that physically defined water hyacinth mats.

Sample Size and Plant Characterization

Total plant length, aerial length, root length, longest leaf mass, plant leaf number, plant mass and blade mass were recorded as plant characteristics.

The sample size of plants was chosen by size constraints and calculations based on sample variance, confidence interval and confidence level (Sokal and Rohlf, 2000). Choice of an adequate sample size to determine various physical characteristics of a mat was complicated because each type of measurement had associated to it a different variance. In other words, sampling was from a discrete distribution.

For example, calculations showed that within size, plant mass was much more variable than plant length. Water hyacinths primarily reproduce through stolons; hence, plants of many sizes are expected in a given size. Indeed, a sample standard deviation of plant mass calculated from a sample size of seven was over half of the average plant mass. Calculations based on this standard deviation indicated that a sample of 400 plants would be needed for a sample mean of plant mass to fall within 0.5% of the population mean with confidence 0.95. On the other hand, calculations based on an observed sample standard deviation of plant length showed that only 15 to 30 plants would be needed to obtain a population plant length within 5% of the true mean length and with 0.95 confidence. To obtain a population plant length within 10%, a sample of 5 to 10 plants would be needed.

In this study, except when otherwise noted, seven plants were characterized for size and component description per size or aggregation. Seven plants permitted an accurate approximation for mean plant length, and the size or fully characterized a size was possible.

Plant Size Distribution and Stolon Length

The best linear square relation to predict stolon length from other plant characteristics was sought, and a classification of plant size based on mass was constructed to compare plant size histograms of different population groups.

Plants from the University of Florida Botany Research Unit, Kissimmee Arm, Lake Kissimmee, and IFAS Edgewater, FL Field Laboratory were characterized according to the general procedure described in the

previous section except that more than ten plants were sampled (see results for mean numbers) and rhizomes were measured. Data from these locations were sampled to obtain data that represent different plant sizes and conditions. The plants from the ten lakes and from Tallmadge had long roots. The plants from Mono Lake were very high, but only cultures were harvested periodically, and they had short roots.

The classification of plant size within size was attempted by this method. First, sample populations of plants were measured to determine if the size data was distributed normally (χ^2 - statistics was calculated). Since this was ascertained, the mean values were attached to plant size classes and their frequency noted. A histogram for each sample population of water hyacinths was plotted using the classes. Means were calculated from each frequency table to check if the distribution of the size classes within each size was normal.

Connectivity, Buryness, and Area Denial

After a physical analysis of water hyacinth mats it was observed that mats could differ by average size of plants, standing density, and buryness and connectivity. Except for connectivity, there were established methods to measure all of the mentioned characteristics, therefore, a method to measure connectivity was devised. Connectivity is a measure of the degree of management through perches and streams. Here connectivity is expected to mean there plants are not touching each other; and the plants are not connected by streams. The principle of the devised connectivity test is that as connectivity increases, more weight/m² and area is required to submerge a mat

Connectivity, buoyancy, and settling density were measured using the following equipment: a 0.74 m² rigid, mesh steel frame, weights and a scale. Measurements were performed as follows:

A mesh steel frame was placed on top of undisturbed water. Weights were placed on the frame until it was just under the water. The weights were removed, frame and net were put away from the surrounding plants, and weights were added again. After each test, plants were characterized, and all the plants within the frame were weighed and counted. The test was repeated several times at the same site on different types of plant mats and trine as followed. The buoyancy was calculated as pressure to submerge the non-connected plants over the area of the frame (P_u) and connectivity as the difference between the pressure to submerge the connected and non-connected plants (P_c). Some mats had similar buoyancy values but different connectivities. To accurately study the effect of connectivity on net stability, the ratio of connectivity to net buoyancy were calculated for every mat tested, and this ratio was termed connectivity ratio (R). The least linear square relations to predict population density and buoyancy from either net characterization were sought.

Results and Discussion

Plant Area Characterization

All but one of plant area data sets for the mats sampled had normal distributions; however, mean values were different. Standard values ranged from -0.2 ± 0.8 to -0.2 ± 0.6 for the normal populations. Standard was 1.0 ± 0.6 for the nonnormal (skewed) distribution. A skewness value of zero signifies a normal population

the standard deviation is given by the square root of six divided by the number of samples (Frazer et al., 2002)

Plant size was represented by six classes and various sub-classes (Table 3.5). This classification system may have to be expanded by the addition of other sub-classes to fit other population groups not sampled. Class 0 represents plants that weigh less than 0.1 kg. This class was subdivided into sub-classes, because the largest and smallest plants in a given population fall within only one adjoining classes (e.g. the smallest plants fall into class 0 and the largest in 1). Class 0 represents large plants that weigh over 0.7 kg. The class structure follows a linear increase in mass until class 3. Classes 3 through 6 allow for greater mass differences because larger and older plants are structurally different from younger and smaller plants. Larger plants usually have more leaves and larger stems, and they have part of their mass in decaying leaves that eventually drop off.

Table 3.5: Plant size classes
Class Subclass

Plant mass
kg

0	0	0.00-0.04
	1	0.04-0.08
	2	0.08-0.16
	3	0.16-0.32
1	1.0	0.32-0.48
	1.3	0.48-0.64
	1.7	0.64-0.80
2		0.80-0.96
3		0.96-1.12
4		0.80-1.20
5		> 0.76

Histograms constructed with these classes were normally distributed (Figures 3.1. - 3.3), and these were distributions close from well-established population groups. Mean averages varied according to age of colonies or habitat. After the data from the one unsymmetrical group (Figure 3.4) was studied, it was determined that the population could be represented by more than one group of plants, one designated by class 0 plants and the others by higher classes (Figure 3.5). This group of plants was not well established because their population was controlled by regular harvests. The histogram illustrates that minor fluctuations would greatly differ from one location to another and even within the same location if the population is still not well established.

Stem Length

Data on stem length from all locations were aggregated and graphed together. Stem length is linearly correlated to plant weight (Figure 3.6); length varied from 1 to 21 centimeters. Two small square equations that relate stem length to plant mass were written: one for plants that showed considerable overall damage or mass concentrated in the rhizome and one for healthy plants with ample foliage.

Area Density

Plant density in this study varied from 0 to 20 kg/m^2 (m^2) and population density varied from 0 to 300 plants/ m^2 . However, an apparent relationship exists between the two types of density (Figure 3.7) - the estimate of population density is the quotient of the biomass density (kg/m^2) at a given aggregation by its average plant mass (average of 7 plants). This simple relation however will be

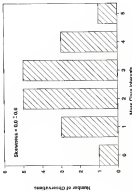


Figure 3.1. Distribution of plant mass in a well-established population. Plants were binned in 1-gram bins, and they ranged 0-500 g.

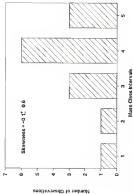


Figure 3.3 Distribution of plant mass in a well-mixed population. The plants were located in Sagar's dam, and were averaged 0.5 kg.

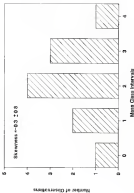
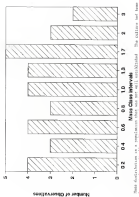


Figure 3.2 Distribution of plant mass in a well-pollinated population. The plants were treated by a full dose of aphidicide. The plants averaged 0.25 g.



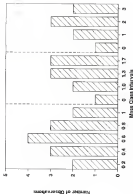


Figure 1.6 Mass data used in Figure 1.4 were redistributed to show that a population that is not right-skewed could consist of many mass groups

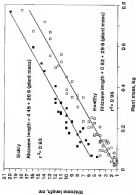


Figure 3.4 Rhizome length is correlated to plant mass. Mass of sickle plants at unconsolidated in rhizomes.

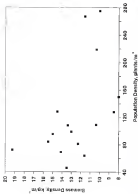


Figure 3.7 The apparent relationship exists between plant and population density of water hyacinth.

populations densities less than 100 plants/m², but it is very unreliable for aggregation of small plants or large populations (Figure 3.8). Small plants usually indicate a population that is not well established, in which case, a reliable value for plant mass can be difficult to obtain (Figure 3.9). A more accurate prediction of plant number was established (Figure 3.9). The relation was developed from Turner's (1965) observation that biomass density increased with petiole length. The least square relation is a nonlinear relation involving the logarithm of average plant height. Biomass density and average plant length are the independent variables. Both of these relations were established for populations with short roots, hence, they should not be used for long-rooted plant systems.

Connectivity and Frequency

Net frequency in this study varied from 20 to 100 % and connectivity from 50 to 100 % (Table 3.1). Peterson and Berlin (1968), on the other hand, found that loading frequency varied from 177 to 450 %. The larger value, 450, probably represented data from an extremely compact aggregation.

Net frequency did not correlate well with standing mass or population density, although frequency appeared to increase somewhat with weed mass density. The relation between net frequency and weed mass density became evident when the data were parameterized by the ratio of connectivity to net frequency (Figure 3.10). The least square relations are presented in Table 3.2. The highest frequency is now given weed mass density occurred when connectivity varied from less than 0.5 and the lowest net frequency occurred when the connectivity

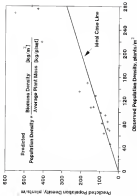


Figure 2.4. Scatter plot of predicted vs. observed population density for the annual plant species using mean average of seven plots. The straight line represents the theoretical case of perfect correlation between predicted and actual population density. Regions of overestimation and underestimation are indicated.

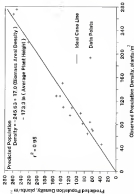


Figure 1.8

This equation predicts average values for population density. Population density depends on area, density and plant length. The straight line represents the theoretical case of perfect correlation between predicted and actual responses.

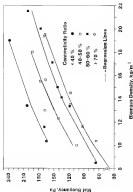


Figure 3.10: Plant biomass is dependent on plant type density, and type and condition of plants (consistency ratio reflects type and condition of plants for a test)

Table 3.1: Values of net baryon connectivity, and assembling ratio

Net baryon	Connectivity	Connectivity ratio	Local density	Average plate length
Pc	Pc	Σ	$\log_{10} \rho$	cm
100-0	32-0	14-0	15.4	16.0
105-0	40-1	26-0	11.9	17.0
90-0	32-0	31-0	13.4	17.0
130-0	60-0	36-0	13.0	21.1
100-0	40-0	40-0	10.0	15.2
140-0	50-1	40-0	12.3	16.0
177-0	70-0	40-0	15.0	53.0
160-0	70-0	40-0	13.0	53.0
177-0	74-0	40-0	15.0	70.0
160-0	70-0	40-0	18.0	16.5
100-0	30-0	67-0	8.0	34.0
104-0	50-0	60-0	13.0	74.1
120-0	70-1	60-0	14.0	60.4
140-0	71-0	60-0	10.0	64.0
133-0	71-0	64-0	17.0	40.1
177-0	90-0	64-0	30.0	40.0
160-0	100-0	60-0	31.5	52.7
94-0	30-0	88-0	8.0	16.3
90-0	30-0	70-0	10.0	31.0
93-0	30-0	74-0	11.0	60.0
110-0	100-0	60-0	20.4	31.0
113-0	60-0	70-0	24.0	40.0
170-0	110-0	70-0	30.4	30.0
100-0	70-0	70-0	10.7	51.0
90-0	140-0	100-0	20.7	80-0

NOTE: The last two entries represent data from long-trusted places

values were greater than 0.7. Net buoyancy is affected by connectivity ratio because the ratio reflects the type and condition of plants comprising a net. High connectivity ratio indicates the plants are supported by petioles entangled or are interconnected by roots, while a small ratio indicates many petioles are flensed and individual plants are supporting themselves. Petioles with flensed plants of low connectivity are common when a culture is periodically harvested, because open conditions lead to flens formation in plants of any size (Box 1982). Low connectivity results when plants are supported. Plants increase net buoyancy, because the specific gravity of a flens is 0.225 or one-fifth the density of a regular petiole.

Table 3.2: Relations between net buoyancy (NB) and standing biomass density (BD)

Equation	Connectivity ratio	r^2
$NB = -0.05 \cdot 0 + 104.5 \ln(BD)$	> 70	0.98
$NB = -0.05 \cdot 0 + 105.2 \ln(BD)$	50-70	0.98
$NB = -0.05 \cdot 0 + 141.2 \ln(BD)$	< 50	0.98
$NB = -0.05 \cdot 0 + 222 \ln(BD)$	< 40	0.98

Net values for buoyancy were collected for long-rooted plants, and they were smaller than those reported for short-rooted plants with similar connectivity and standing densities. This is expected because

long-rooted plants have several 20% more biomass underground the water table compared to short-rooted systems.

Summary

Water hyacinths consist primarily of roots, and although complex structurally they can be defined by leaves, rhizomes, roots and stolons. Flowless petioles help support the plant under dense conditions by entanglement with leaves from other mats. Rhizome length is correlated to plant mass, and root length to nutrient availability.

Established populations of water hyacinths have normally distributed plant mass, while aggregations of small plants or plants subjected to regular harvests can consist of many types of plant groups.

Population density is correlated with plant length and standing biomass density.

Connectivity and net buoyancy are independent properties that define the mechanism of the vertical stability of mats and plants.

CHAPTER IV SYSTEMS AND ELASTIC PROPERTIES

A system is a collection of interacting elements for which there are cause-and-effect relationships among the variables. A water hydraulic system may be subjected to friction and elastic forces resulting from interactions of pistons with each other, external driving forces of steel or testing devices, and drag from the interaction of pistons with water. Therefore, probable system elements are related to body forces (internal and gravitational) and surface forces (pressure and shear). To represent the elastic and frictional forces in form of system elements, constitutive equations are required. They depend only on the characteristics of the physical material that comprises an element and on its geometry, and they describe basic relationships between two variables (e.g., Hooke's law for solids and Newton's viscosity law for fluids).

Relationships Between Forces

The purpose of this section is to develop the constitutive relationships between drag and testing velocity as a function of size of an aggregation of water hydraulic and plant size and shape.

Review of Literature.

Spheres, cylinders and some patches of minor irregularities are submerged in water. When they are placed in flow, they are subjected to drag. Drag is composed of two components: friction drag resulting from tangential stresses caused by friction of the fluid against the interface, and pressure or form drag, resulting from pressure forces exerted by the fluid normal to the surface of the body. The latter is predominant at high Reynolds numbers. Drag on a body can be determined analytically for simple shapes and flows and experimentally for slightly more complicated geometries, but it is usually determined experimentally for bodies in turbulent flow. Customarily, the formula for overall drag is written in the following form:

$$D_f = \frac{C_D A \rho V^2}{2}, \quad (8.3)$$

where D_f is drag force, A is a the area of the body projected in the direction of flow, ρ is density of the fluid, C_D is a drag coefficient and V is the velocity of the fluid relative to the body which may move or be stationary.

A similar drag formula was initially proposed by Newton, but the original proportionality factor (instead of C_D) did not always coincide with experimental data. This problem was resolved when a function of Reynolds number was used to replace the original proportionality factor (Proudman and Thomas, 1934). Reynolds number aided calculation of drag in a Newtonian fluid on slender bodies which have the same orientation with respect to the free stream velocity. The relation between C_D and Reynolds number must be

determined for turbulent flow only experimentally. Furthermore, experiments proved that when the total drag on a body consists exclusively of pressure drag ($i = 0$ for high Reynolds numbers), the drag coefficient is large and independent of Reynolds number. In the case of friction drag, the drag coefficient is a function of Reynolds number, and it is usually small in value.

Rhinoids are the principal projections into water. They are cylindrically shaped. Due to the shape of rhinoids, water hydrodynamics in flow can be modeled from flow analysis for a series of cylinders (e.g., in a bank arrangement). Also, information concerning the flow properties of water hydrodynamics can be ascertained from a review of studies of flow hindrance by aquatic plants, which are assumed to represent roughness elements. These topics will be reviewed next.

Flow around a bank of cylinders. Since the shape of rhinoids can be assumed to be cylindrical, the following information on resistance over a bank of cylinders is pertinent to the description of water hydrodynamics behavior. Grinstein [187] investigated shear stress, τ_s , in open flow over:

$$\tau_s = \frac{F}{g} \frac{U^2}{D} \quad (5.1)$$

where g = aquatic area flow, g is the number of major restrictions introduced by flow through a bank, and F is a friction factor which is a function of Reynolds number and the ratio of transverse to longitudinal spacing.

It found that resistance coefficients for different arrangements of cylinders were functions of Reynolds number (based on cylinder

diameter), the number of major restrictions encountered in flow through the bank, and the ratio of transverse to longitudinal spacing. The number of major restrictions depended on the configuration of the cylinders: for inline arrangements, p equaled the number of rows; for staggered arrangements, with minimum flow area in transverse intermediate spaces, p equaled the number of rows, and for staggered arrangements with minimum flow area in diagonal intercylindrical spaces or openings between cylinders, p equaled the number of rows times two. Generally, the friction factor was constant until Reynolds number exceeded 10,000. Above 10,000, the general characteristic curves for friction factor declined downward.

Later, the equation for total head loss due to friction was refined to resemble the friction factor from a Moody diagram (Equation 4-4) (Boys and London, 1964 and Robinson and Parker, 1962):

Total head loss (Boys and London, 1964):

$$v_h = F \frac{A}{A_n} \left(\frac{V}{V_0} \right)^2 \quad (4-5)$$

where F is Moody friction factor, V is mean velocity, $\frac{A}{A_n} = \frac{4R}{H}$,

A is total friction area, A_n is total flow cross sectional area, and H is flow length. Both A and H are measured from the leading edge of the first cylinder row to the trailing edge of the last row of cylinders, and R equals the hydraulic radius (hydraulic radius for flow normal to cylinders is cylinder diameter).

Resistance elements and turbulent resistance Hydraulic resistance is often presented in an alternative form that was derived by dimensional analysis [Lash, 1980]:

$$\Delta p_T = \frac{\rho}{2} + \left[\frac{\rho}{\eta^2 x} \right]^m V^2, \quad (4.3)$$

here x is a characteristic length, η is kinematic viscosity, V is average velocity in a pipe section and m is an exponent obtained through experimentation. Reynolds number is defined by $\frac{Vx}{\eta}$. For $m=0$, the formula that is often used to determine for the case of turbulent flow through pipes is obtained. It is similar to Equation 4.2. Lash [1980] suggested that the value of m appears to depend on the roughness of a surface.

Nikuradze's empirical studies of flow resistance by aquatic plants have assumed that plants in channel flow act as roughness elements. Basic ranges of flow regimes for roughness elements have been established on the basis of height and spacing of roughness elements [Schlichting, 1955]. There are 1) the hydraulically smooth regime where the size of the roughness is so small that all protrusions are contained within the viscous sub-layer, 2) the transition regime where protrusions influence the viscous sub-layer, and the resistance compared with a smooth pipe is due mainly to form drag experienced by the protrusions in the turbulent boundary layer, and finally, 3) the completely rough regime where all protrusions extend outside the viscous sub-layer and the resistance is due mainly to form drag. These regimes are determined by the following criteria:

a. Hydraulically rough,

$$\frac{k_g V_m}{\nu} \geq 50, \quad F = 0.045 \{ \text{Reynolds number} \}^{-1/4}$$

b. Transition

$$0 \leq \frac{k_g V_m}{\nu} \leq 70, \quad F = 0.045 \frac{k_g}{\nu} \{ \text{Reynolds number} \}$$

c. Completely rough

$$\frac{k_g V_m}{\nu} \geq 70, \quad F = 0.045 \frac{k_g}{V_m}$$

where k_g denotes the grain size in Einstein's sand roughness, V_m is the friction velocity defined by $\sqrt{\tau_0/\rho}$, R is the hydraulic radius, $\frac{k_g V_m}{\nu}$ is often referred to as the wall Reynolds number, and ν_m is shear stress at the wall. Friction factor (Darcy-Weisbach) is related to shear stress by $\sqrt{\tau_0/\rho} = \sqrt{2R} F$. Furthermore, k_g has been related experimentally to Manning's n (Strickler, 1959)

$$n = \{1/25\}^{1/3} k_{g[25]} \quad (4.4)$$

with value $n^{(2/3)}k_g^{-1}$, and $k_{g[25]}$ is roughness in meters such that 0.02 of the material is of lower size. The equation is valid for $n \leq \frac{8}{V_m}$ (6000). Manning's n is a roughness coefficient; its value increases with increasing roughness.

In reference to flow over aquatic plants, Ren and Palmer (1976) demonstrated by experiments that the degree to which bottom channel vegetation restricts water flow depends largely upon the degree to which vegetation is bent and flattened by the flow, which in turn depends mainly upon physical characteristics of the vegetation—the manner of growth and the velocity and depth of flow. The plants were different kinds of grasses and lilies. More specifically, when flow was sufficiently deep and fast to bend over and submerge the channel

vegetation, a greater velocity resulted due to a greater slope of the energy grade line resulting in lower resistance to flow. Manning's n values for clean and well-sorted vegetation appeared to decrease to a nearly constant value of about 0.05 as depth of flow was increased or over the plane. Values of n for different silt plane species varied widely for all depths of flow. In regard to population density for a specified flow depth, a dense cover of vegetation (200 plants/m^2) offered twice as much resistance to flow as a less dense cover (17 plants/m^2). The authors developed a graphical method for the determination of channel cross section. To do so, they plotted Manning's values versus VR (velocity times hydraulic radius) because they perceived resistance to be a function of the degree of flattening of the vegetation, which is influenced by velocity and depth of flow. In a nutshell, they found that well-silted vegetation channels lining become well flattened, the value of VR influenced the degree of resistance the lining offered. However, when the vegetation was green, or nearly so, and well submerged, it became practically constant and seemed to correlate with VR.

On the other hand, Brown and Day (1973) determined the values for the local friction factor, one for street and the other for grass regions. They found that friction for the street and parking regions was a function of a relative roughness factor, while for grass plant roughness, friction factor was a function of VR. The experimental results were in the form of graphs. This apparent discrepancy in the two investigations could relate to the manner the vegetation was submerged or to the depth of flow. Although, it appears that Brown and Day were correct in their conclusion because 1) flow theory of

a fully rough regime is a channel predicts the drag coefficient to be independent of Reynolds number, so drag varies with U^3 and is independent of viscosity, and (2) crown vegetation is more likely to cause a fully rough flow regime because the roughness elements are bigger to provide larger values for the wall Reynolds number.

Many investigators have found that Manning's n or friction coefficient increases with the amount of aquatic biomass present (Peters, 1960; Komen et al., 1990; Paoletti, 1994; Petryk and Benington III, 1995; Rall and Rhee, 1992 and Stephens et al., 1993). Petryk and Benington III (1995) defined density of aquatic vegetation by the vegetative area per unit length of channel per unit area of flow as expressed by the following equation:

$$\text{Density of vegetation} = \frac{A_v A_f}{A_f d}$$

where A_v = projected area vertical area of vegetation in the flow direction in a channel of length d and cross area A_f .

The effect of plant density on flow resistance can be related to roughness spacing. Paoletti (1992) studied longitudinal spacing of strips in a channel. He found that the ratio of roughness spacing to roughness height was an important determinant of maximum relative roughness, and it was constant regardless of the value of the roughness width to height ratio. Sayre and Sutherland (1987) studied both longitudinal and transverse roughness spacing, and he used dimensional analysis to develop the following spacing parameter:

$$\text{Spacing parameter} = \frac{3b}{u(x + k)} \quad (4.5)$$

where x = longitudinal spacing of roughness baffles, k = roughness

height, b = roughness width and s = average spacing length. The flow was considered to be in the completely rough range. Data were analyzed in terms of the Karman-Prandtl concepts of turbulent flow near a rough boundary, and the following equation was fitted to the data:

$$\frac{U}{\sqrt{g}} = 5.08 \log \frac{U^2}{g h} + C_2$$

where C_2 is the drag discharge coefficient = $K^{1/2}/s$, h is normal depth, C_2 is an experimental constant dependent on roughness spacing, b is height of roughness element, and g is acceleration of gravity. Similarly, using the equation for logarithmic velocity profile near a turbulent wall, Rouse et al. (1950) obtained this equation for flow velocity in a vegetated channel:

$$\frac{U}{\sqrt{g}} = C_1 + C_2 \ln \frac{A}{A_0}$$

where C_1 and C_2 vary with the vegetative density and the flexibility of the vegetation, A is the gross cross sectional area of the channel and A_0 is the area of vegetative cross section.

In summary, the most relevant concepts discussed in this section for application to water hyacinths are:

- Theory and experiments have shown that bigger plants and denser vegetation produce a larger drag coefficient.
- Empirical relations for pressure drag across banks of cylinders in cross flow have shown that cylinder diameter and size of

the bulk specifically, cylinder length, bulk length and bulk width influence friction factors.

c. For Reynolds number (based on tube diameter) less than 10,000 the friction factor of a bank of cylinders in cross flow is independent of Reynolds number.

d. Transverse and longitudinal spacings of cylinders in cross flow and of roughness projections in flow affect drag coefficient.

e. Plates that are flexible in flow offer less resistance to flow than most plates.

f. Experiments have proven that when vegetation is present the drag coefficient is independent of Reynolds number, and it is dependent on a relative roughness parameter.

Drug Model for Porous Structures.

In this section, an expression for drug valid for a rectangular aggregation of plates of any porosity density or spacing and rotational area in flow is developed in this section. Neither internal nor convective velocities appeared desirable for the problem of water absorption in steady flow due to the assumptions of (1) constant and zero mean turbulence, and (2) the wall deforms when moved. Therefore, an analytical solution based on rigid, rough flat plates is not appropriate. Consequently a drug model was developed based on Burton's drug equation (Equation 4.0), and drag coefficients were obtained experimentally.

To begin, a model for drug must be dimensionally correct. The drug equation, Equation 4.0, was used after it was modified by the incorporation of the number 2 into the drag coefficient:

$$D_f = \rho \cdot A \cdot C_d \cdot V^2 \quad (4.6)$$

where D_f equals the drag force (Newtons), ρ equals the density of water approximately equal to 1000 kg/m^3 , A equals the projected area to flow (m^2), C_d equals a nondimensional drag coefficient, and V is the testing velocity (m/s). Although Equation 4.5 implies C_d is a function of Reynolds number, evidence in the literature indicates that the drag coefficient for an erect, rigid propeller 1/4th a diameter of water squirrels, is independent of Reynolds number. If indeed, C_d of water squirrels is independent of Reynolds number, the testing velocity would enter the model only as V^2 . In substitute this, the Reynolds number was calculated. (Diameter of blumes range from 0.01 to 0.03 m. The Reynolds number for water squirrels based on a 0.01 m diameter and a testing velocity of 0.4 m/s is calculated below.)

$$\begin{aligned} \text{Reynolds number} &= \frac{\text{Diameter of Blume} \times \text{Testing Velocity}}{\text{Kinematic Viscosity}} \\ &= \frac{0.01 \times 0.4}{0.804 \times 10^{-6}} = 4975.0 \end{aligned}$$

However, for 0.03 m diameter, the Reynolds number is approximately 15,000 at 0.4 m/s, which was usually the observed upper limit of stable testing operations. Medium and small blumes can be expected to have Reynolds numbers below the lower limit at which drag of cylinders is where flow is dependent on Reynolds number.

To use the drag equation, the friction area must be defined. The total friction area was assumed to approximately equal the projected

area of a rhizome (A_p) multiplied by the total number of plants (N_p) in an aggregation or

$$A = N_p \cdot A_p \cdot L_T = N_p \cdot A_T \quad (4.7)$$

A_p equals the average diameter of the rhizomes and L_T , average length.

The shape of a water hyacinth unit also influences the drag coefficient, since drag coefficients are only similar for geometrically similar bodies of the same orientation to velocity in the flow stream velocity. Therefore, a unit which is wider or has more plants along the flow direction will have a different drag coefficient than a narrower unit. The shape factor enters the cylindrical equation twice as μ (Equation 4.4) and nondimensionally as A_p/A_T (Equation 4.5). For a rectangular unit of water hyacinth, the simplest nondimensional shape factor is the ratio of the two dimensions, i.e. width B , to length L .

For this study the transverse and longitudinal spacings of water hyacinths within a unit are assumed uniform. As a result, the drag coefficient was considered to be a function of a shape factor and, possibly, Reynolds number. Then, the proposed drag model for water hyacinth becomes

$$D_T = \mu \cdot N_p \cdot A_T \cdot L_T \cdot C_D \cdot V^2 \quad (4.8)$$

where,

$$C_D = f \left[\frac{N_p \cdot \mu}{B}, \frac{B}{L} \right]$$

This proposed equation is valid for any rectangular aggregation of water hyacinths without the need for graphs or tables to determine C_d ; provided, of course, that C_d is obtained experimentally.

Materials and Methods.

This section presents the procedures that were used to 1) test the hypothesis that the drag coefficient of a water hyacinth mat is independent of Reynolds number and 2) determine the empirical relation of C_d to mat geometry.

Plant material.

Aggregations of water hyacinths composed primarily of small, medium or large-sized plants, were tested. Small plants averaged 20 cm in length; medium plants averaged 51 cm; and large plants averaged 80 cm. Because the water was deep enough, a buoyancy test was performed on the mat before it was moved to the testing area. Water hyacinths were obtained from the University of Florida Spring Research Unit, Silver's Arm, and Lake Alton. The lakes are located near the University of Florida. After completion of each experiment, water plants were measured for stem, rhizome length and diameter, largest petiole length, plant length and root length. Root length varied according to habitat and ranged from 3 to 24 cm. The presence of nodules and floats was noted.

Aggregations of plants from the Silver Arm were returned to the testing area in this manner. First, a 3.66 by 1.22 m area of plants was isolated, then, a frame of wood and fence wire was positioned underneath the plants. Lifting the frame removed the plants from the water in their natural position. Because the water was too deep to

the test tubes, the hydrometers were dropped to shore and hand positioned on the frame.

Tested apparatus.

To measure the drag coefficients of water hydrometers, towing experiments were performed on different sizes of rectangular approximations of water hydrometers. The tests were performed in the Hydrostatic Lab, Civil Engineering Department, University of Florida. The following equipment were used: an adjustable frame towing, floating frame-like structure, a powered winch, water velocity meters and instrumentation (Figure 4.6).

The floating structure consisted of two frames, an outer supporting structure of $36 \times 36 \times 1.5$ m square, steel tubing on two 180 m PC pilings and an inner containment frame of $25 \times 25 \times 2.1$ m square, steel tubing. The inner frame weighed approximately 15 kg and was suspended from the outer frame by four reinforced carbon fiber instrumented with temperature-compensated, bonded strain gage bridges. The elevation of the outer frame above the piers was incrementally adjusted so that the lower frame just cleared the water. Arrangement of the two frames ensured that water hydrometers contained in the lower frame could not touch the upper frame. The lower frame was fitted with 8 m red lines projecting 30 m below and 150 m above the frame bar to prevent piers from striking over or reeling under the frame, and, it was arranged to be subdivided into 2.4 m long \times 1.2 m wide \times 1.8 m \times 1.2 m, 1.2 m \times 1.2 m, 1.2 m \times 0.6 m, and 0.6 m \times 1.2 m containment areas. The 0.6 m \times 1.2 m containment area had lines placed with several

Figure 4.4: Tuning apparatus included a Tread-Tite® (Highway, Inc.) mounting device, and a cooling



it because the plants escaped from the sides during preliminary experiments.

The length of each chamber beam was 0.30 m and the width and thickness were 0.06 and 0.02 m, respectively. Two 330-ohm strain gauges was attached to each side of a chamber beam. hence, two four-gauge bridge circuits, one for each end of the beam, were used to measure the forces against the beam. The force measuring system was calibrated by using a spring scale and by recording the corresponding voltage (Figure 8-1).

Flowing velocity was recorded by two methods. Propeller type velocity meters (4-400 Rpm) current meters) attached to the trailing, were used in the initial tests. After the meters failed to function properly, distances along the towing path were marked off and time to travel a distance was recorded through the aid of video equipment.

The force and velocity signals from the two rigs were recorded by a Campbell Scientific C2P data logger with observations recorded every second.

Before each of the tests, a grid of thin nylon wires was placed over the set of plants for subsequent observation of plant movement in relation to each other. The tests were videotaped. Some underwater observations were visually recorded through a glass window in the side of the chamber.

A typical experiment proceeded as follows: Plants were placed into the experimental frame. The frame was pulled by a trolley through a 2.4 m wide channel with the water depth kept at 0.50 m. Plants were

Figure 4.1. Styria pupae were attached to oscillating frames which were suspended from the insect frame and attached to the lower frame. The lower frame contained plants. Rooting velocity was measured by a current meter.



tested at a constant speed for a distance of approximately 11 m. Testing speed was increased from 0.1 to 0.6 m/s until the plants started to roll under the aerodynamic structures. When this happened, the test was ended, and the sample was subdivided. Some of the plants were removed to check whether and when. Removed plants were desiccated and weighed to determine the quantity of biomass in the sample. Testing tests were repeated as approximately similar tests until all test sites were tested.

Analysis of data.

Force and velocity data for each test were regressed according to the equation

$$D_T = C_D V^2 \quad (4.5)$$

where D_T equals total drag force in Newtons, C_D is a coefficient of proportionality equal to the product of $(\rho R_p D_p L_f C_{dp})$ from Equation 4.3 and γ equals testing velocity. Drag forces was determined to be the force on the container bases after the plants had stopped accelerating and the forces due to the acceleration stage of the test had dissipated to a slight fluctuation. The parts of the testing tests were labelled the unsteady and steady stages (Figures 4.2 and 4.3).

The data were analysed for error. Besides an experimental error, can be quantified when the distribution approaches a normal distribution (Meadley, 1998). The principal source of experimental error was the data logger. Error of the end point of the data logger was ± 0.003 m, which translates into an error of ± 0.008 Newtons of the force data during the steady state condition of a testing test.

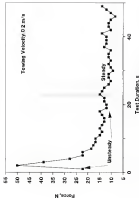


Figure 8.2 A wire specimen was tested at 0.2 m/s. Testing time (measured to the nearest 0.1 s) is the steady portion of the curve.

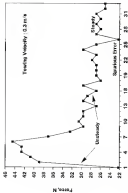


Figure 4.3

A steady laparoscope was run randomized slowly in a fluid velocity of 0.2 m/s. Steady-state errors were found to test data. Steady and unsteady portions of the curve are marked.

was calculated for two runs (testing velocities: 0.2 m/s and 0.3 m/s) and the values were $+0.262 \pm 0.018$ and $+0.35 \pm 0.020$. If shown to have the distribution is normal, accordingly, the force distribution can be considered to be normal. A histogram of the drag force distribution is shown in Figure 4.4. The dispersion of drag force about the time average was obtained as standard deviation when the distribution was normal. Standard deviation of drag force, σ_F , was calculated using data from the two tests, while standard errors of \bar{R}_F (Equation 4.10), drag force calculated from the regression model, and of the coefficient \bar{C}_D (Equation 4.11) were computed for every test. Since the International Standards Organization has recommended that the probability level used should be at the 95 percent level (Burchey, 1976), error was reported using the corresponding t statistic multiplying either the standard error or deviation.

Standard Error of Estimate, \bar{R}_e (Burchey, 1976),

$$\bar{R}_e = \left(\frac{1}{N-1} \sum d^2 \right)^{\frac{1}{2}} \quad (4.10)$$

where d is the deviation of the actual value from the computed value taken from the curve of relation or $\bar{R}_e = \bar{R}_F^{\frac{1}{2}}$ and N is sample size.

Standard Error of the Coefficient (Burchey, 1976) is,

$$\bar{R}_e = \frac{(N-1) \bar{R}_e^2}{(N-1) \sum x^2 - (\sum x)^2} \quad (4.11)$$

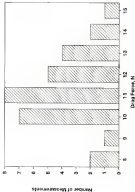


Figure 3.4 Histogram of drug data obtained from the acute portion of a testing unit. Testing including one 8.3 mg, Barbitol followed a normal distribution.

Spurious errors were discarded (Figure 4.10) when data lay more than α standard deviations from the sample mean. The value of α depends upon the sample population size (Benedict, 1986). For example, for a population of 100, α equals two.

Systematic error in the flow costs could be related to the design of the test frame. Three circular plexiglas within the test frame. The intention is that this was considered small, because the diameter and number of tubes were small relative to those of the rhinopneum. However, the drag of tubes on all but the 1.22 m long by 0.3 m wide test frame, which had seven tubes, was measured as 1.2 N at a towing velocity of 0.5 m/s and 3.0 N at 0.8 m/s. The drag on the smaller frame was nearly 20 % higher than the drag on other frames. Thus are required in any towing system, so their exclusion from the experiments was not deemed necessary. Also, force measurements could be susceptible to systematic error if the calibration of the strain-gages was inaccurate. The gages were calibrated twice using different input voltages: spring scales and weights of force, and the calibration factors remained the same. Finally, another source of systematic error is the number of significant digits the data logger uses to store force readings. Ten bit and very small forces would not be recorded accurately by the data logger.

Results and Discussion.

Experimental observations.

At the beginning of a towing run, plexiglas tended to bunch together, slowing the length of the run to decrease. Despite all the movement of grid lines on top of the plexiglas demonstrated mechanisms of cooperation. When a trial began, the grid lines at the leading edge

of the net reached stationary state at the test speed. Once the state had reached constant speed, the grid lines were equally spaced again but closer due to compression of the net.

Dragation increased with towing velocity until "roll-under". "Roll-under" occurred at a velocity averaging 0-45 m/s. At that point, the plates on the leading edge started to roll forward under the water. Once under water, the plates tended to flexion and underneath the other plates sitting on the surface. In this way, the plates on the surface passed over submerged plates and, in turn, rolled under them they were positioned at the leading edge (Figures 4.3-4.5). In "roll-under", the force on the cantilevers greatly increased, and the portion of the force - velocity graph pertaining to "steady" was absent (Figures 4.6 and 4.8).

Some towing tests were repeated in an attempt to duplicate the results. Each time repeated tests fell on the model line (Figure 4.8); hence, the tests could be duplicated under similar conditions.

When plates are stationary, their force being decreased in the water, yet, they are very flexible. Subsequent observations showed the rods to be completely prone at a towing velocity of 0.33 m/s and waving back and forth at lower speeds. Once the rods were prone, the component of drag was due to resistance.

Review of regression analysis

A model fit experimental data provided the standard deviation of the dependent variable both in the range of the computed standard error of the predicted value. To determine the efficacy of the proposed model, cross average values of the drag force were plotted against the square of the towing velocity for two sample tests. The

Figure 6.5 When eggs were found, they swarmed. Amount of competition estimated with feeding velocity, at 2.1 m/s, competition or might.



Figure 4.4 The cat was reared at 8.1 Hz/m, and is conspecific approximately 20 cm.



Figure 4.3 The red sun moves at 0.4 m/s , and is suspended approximately 10 cm



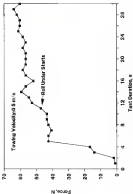


Figure 4.8 is "ball under", rising on the leading edge started to roll forward under the wire. Testing force increased, and the position of the force a velocity graph providing in "steady" was shown. "Ball under" occurred at velocities exceeding 0.5 m/s.

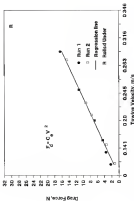


Figure 4.3 Drag force can be determined under similar conditions up to the point that the sub starts to "pull under"

regression equation (Equation 4.6), the corresponding values of standard deviation (shown as SD), and error (shown as SE) were plotted (Figures 4.10 and 4.11). For the two tests analyzed but excluding data for the largest towing velocity, the ranges of the standard deviations fell in the range of the standard error (SE confidence level). The fact that the ranges of the standard deviations of C_D at "roll-over" velocity did not fall between the ranges of the standard error shows that the flow behavior of the water jetpack changes near "roll-over".

Values of proportionality factor, C_{D0} , (Equation 4.6) were computed and errors are expressed to two significant figures (Table 4.6). In two tests, percent relative error was greater than 100%, whenever the number of data points was less than five. Indeed, these samples also had not reached because some samples reached "roll-over" very quickly. In these tests, few data points were obtained. However 80 percent of the C_{D0} values had less than ten percent relative error and are consistent in each experiment and between experiments. Values of the coefficient, also, appeared to vary according to physical variations in the jet and individual pilot structure.

Low-Coefficient

Provided that Equations 4.6 and 4.9 accurately describe drag for any choice of pilot and jet size and that C_{D0} is not a function of Reynolds number, experimental drag coefficients should be nearly given by

$$C_D = \frac{C_{D0}}{1 + \frac{R}{L} \frac{A_p}{A_j}} \quad (4.10)$$

where $C_{D0} = 1 \left(\frac{R}{L} \right)$

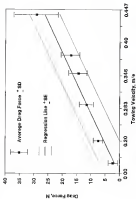


Figure 4.10: Drag inside channel. Line of regression and error. Boat was towed at 0.20 m/s ± 0.03 m/s.

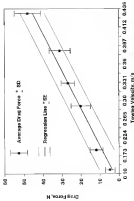


Figure 4.11 Error bounds about line of regression and drag. Test wire was 3.46 m long \pm 1.21 m wide.

The values of C_{Dj} obtained by this expression from C_D are tabulated in Table 4.1 along with values of the shape factor and other variables. Variables R_p and L_p were calculated from expressions developed empirically in Chapter 3. Release diameter is difficult to measure due to the presence of tails, but, since they have been shown to vary little within a population or by as little as 0.5 cm, the following classification scheme was used: shell plates have 1 cm diameters, middle plates, 2, and tail plates, 3 cm.

To assess the accuracy of the experimental drag coefficient, experimental error of Equation 4.15 was calculated by the following equation (4.16) (Bendat, 1978):

$$E_{C_D} = 4 \left(E_C^2 + E_T^2 + E_{R_p}^2 + E_{L_p}^2 + E_{C_{Dj}}^2 \right)^{1/2} \quad (4.16)$$

where E_{C_D} denotes the percent error in the drag coefficient contributed by the i -th parameter. The error for each estimated value of C_{Dj} is shown in Table 4.2, and its average is approximately 10.8. Density of the water, ρ , could vary with the quality of water (dirty or clean) and temperature, but that error was assumed to be within 0.5 percent. The errors due to the estimation of R_p and L_p were assumed to be 10% and 2% respectively since that is approximately the error introduced by the empirical expressions of Chapter 3. The error due to C_D was estimated to be 10%. After substitution of these values of percent error into the error equation, the percent experimental error for the drag coefficients was 15%.

Table 4.1 Orog contribution for different soil generation and other soil characteristics

$$C_d = \frac{C_p}{1 + \frac{C_p}{A_p}}$$

C_p	C_d	A_p	Total porosity of plastic	Width to length ratio	Flow area	Boundary on C_d	
Experiment 1							
200 G	0.73	328	0.5	0.5	0.00-1	High	
200 G	1.53	248	1.0	1.0	0.00-1		
100 T	1.18	128	0.5	0.5	0.00-1		
Experiment 2							
200 G	0.61	338	0.5	0.5	1.00-3	Low	
200 G	1.13	204	1.0	1.0	0.00-1		
100 G	1.40	133	0.5	0.5	0.00-1		
78 G	0.68	133	0.5	0.5	0.00-1		
Experiment 3							
200 G	0.60	376	0.5	0.5	0.00-1	Low	
200 G	1.30	203	1.0	1.0	0.00-1		
100 G	1.40	119	0.5	0.5	0.00-1		
67 G	0.63	119	0.5	0.5	0.00-1		
Experiment 4							
200 G	2.00	147	1.0	1.0	0.00-1	High	
100 G	1.43	90	0.5	0.5	0.00-1		
60 G	0.61	90	0.5	0.5	0.00-1		
Experiment 5							
100 G	0.74	613	0.5	0.5	0.00-1	High	
60 G	1.75	308	1.0	1.0	0.00-1		
118 G	2.65	180	0.5	0.5	0.00-1		
61 G	0.68	180	0.5	0.5	0.00-1		
Experiment 6							
200 G	0.60	74	0.5	0.5	1.00-3	High	
100 G	0.67	54	0.67	0.67	0.00-1		
100 G	1.30	10	1.0	1.0	0.00-1		
101 G	0.37	14	0.5	0.5	0.00-1		
68 G	1.00	18	0.5	0.5	0.00-1		
Experiment 7							
200 G	0.73	107	0.5	0.5	0.00-33		
200 G	0.73	309	0.67	0.67	0.00-1		
100 G	1.40	74	1.0	1.0	0.00-1		
100 G	0.35	35	0.5	0.5	0.00-1		

The functional relation between C_D and slope factor W/L was ascertained. A regression equation relating C_D to W/L was determined to be nonlinear (Figure 4-12), and is written below.

$$C_D = 0.408 \log(0.006 \frac{W}{L}) \quad (4-18)$$

Not all values of C_D were used in the formulation of this relation because they were deemed to be outside of 95 percent confidence intervals. Confidence intervals of C_D were established for every slope factor $\frac{W}{L}$. The outliers were basically from two groups of test slices. Four outliers originated from an area of 1.00 in long x 0.6 in wide, and one outlier from an area 0.6 in by 1.00 in. The former values were results of systematic errors. They were related to facts resulting in the lowest drag values. As was mentioned before, these low forces may have been recorded improperly by the data logger. The offset value of the data logger added to the value of the real force. Two attempts were made to correct this problem. One method, which resulted in reasonable values of C_D (Equations 5 and 16), was to supply the input signal from the strain gages positioned on the upstream face. The second method, which involved supplying a higher voltage to the strain gages, was unsuccessful.

On the other hand, the two outliers from the other sample size could not be explained until after videotapes of the experiments were viewed. The plates were seen bending in the flow. The bending action increased friction drag over pressure drag, therefore the drag coefficients were lower than reported (Schlichting, 1979).

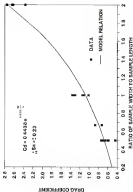


Figure 4.12. Drag coefficient of a square baffle and its a function of end geometry

The regression equation relating C_d to Re was analyzed for experimental and dispersion errors. Dispersion of the data around the regression curve is given by ΔC_d where t is the t statistic (95% confidence level) and S_{reg} is the standard error due to the regression. If the regression equation is valid, the confidence intervals of C_d for every slope factor $\frac{\partial}{\partial C}$ should fall within the range of C_d given by the standard error of the regression. This was true except for Re equal 2 (Figure 4.12). However, the discrepancy is negligible.

Summary

-Hydrodynamic drag coefficient of water hyacinths is independent of Reynolds number but dependent on the shape of a net.

-The water hyacinth drag equation was based on Darwin's drag formula, and it included the drag coefficient, projected area and number of rhizomes and the square of velocity.

Elastic Characterization

Elastic deformation of water hyacinths occurs when an external force applied on a net compresses plants against each other. Bases of the plants in contact during compression have been observed to be the rhizomes. The objective of this section is to document experimentally the elastic modulus and Poisson's ratio of the rhizome. Literature Review.

Intensive study of the rhizome reveals that it is a composite material of two or more components having various, usually contrasting physical and mechanical properties. Water hyacinth rhizomes, like other natural composites such as scales of plants or bones of animals,

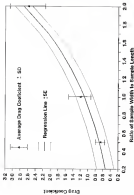


Figure 4.12: Linear trends about bias of regression and drug coefficient

have radial structure (Thompson and Eisele, 1981).

Micro-structurally, the rhizome consists of four regions: outer region covered with profusely-branched roots of 0.4 to 1.5 mm diameter, epidermis, cortical region and stolon. Rhizomes vary in length from approximately 1 to 20 cm and in diameter from approximately .mm to three millimeters.

Rhizome properties are measured in very principally in the radial direction because of the symmetrical shape and radially varying structure of rhizomes. The elastic properties of a cross cut are similarly characterized and have been extensively studied (Ginsburg, 1983; Ginsburg and Chikaraishi, 1983; and Chikaraishi and Ginsburg, 1984). The methods and equations from investigations used were adopted in this work to determine the elastic response of water hyacinths in tension of their cylindrical rhizomes.

Equations for the determination of elastic modulus E and Poisson's ratio ν of rhizomes under quasi-static radial compression between two steel flat plates using a large deformation approximation, are (Chikaraishi and Ginsburg, 1984)

$$E = \frac{2.7(1-\nu^2)(\Delta C)^2 - 3.6P + 37.7^2 \Delta C \Delta C - 36.7^2 \Delta C \Delta C}{(1 - 0.7^2 \nu + 7.7^2 \Delta C)} \quad (4.10)$$

where P is the known experimentally applied force per unit length of the cylinder. Poisson's ratio ν is calculated

$$\nu = \frac{(\Delta C \Delta C - 1) - 3.6P + 37.7^2 \Delta C \Delta C - 36.7^2 \Delta C \Delta C}{(2 \Delta C \Delta C - 1) + 3.6P + 37.7^2 \Delta C \Delta C} \quad (4.11)$$

w is the deformation of the diameter, and section of the cylinder,

w is the normal approach of the flat plate to the center of the cylinder, or half of the total normal deformation of the cylinder,

$$k = \ln(4/3\pi) = 0.5$$

Γ is the half-angle of contact between the cylinder and the flat plate which can be calculated by the following equation:

$$\eta_1^2 = \frac{[(1+\nu_1) - 0.5(1 - \nu_1^2)\alpha - \nu_1^2\gamma(1+\alpha)] + 0.5\gamma^2\alpha\beta - 0.5\gamma^2\alpha\beta\gamma(1+\alpha)}{[2\gamma^2 - 0.5\alpha + 2\gamma^2\alpha\beta - 0.5\gamma^2\alpha\beta\gamma(1+\alpha)]}$$

where $\gamma =$ radius of the cylinder:

$$\eta_2 = \frac{\gamma^2(1 - \nu_2\gamma^2\alpha)}{\alpha(1 - \gamma^2\alpha + \gamma^2\gamma(1+\alpha))} \quad \text{and}$$

$$\eta_3 = \frac{\gamma^4(1 - \nu_3\gamma^2\alpha)}{\alpha(1 - \gamma^2\gamma + \gamma^2\gamma(1+\alpha))}$$

Values from these equations which are based on a large deformation theory, were compared to values based on the Hertz linear elastic contact theory and it was concluded that the large deformation theory was the most accurate but only by one percent.

When two cylindrical bodies, with their axes parallel, are pressed in contact by force F per unit length, they make contact over a long strip of width $2d$ lying parallel to the axes. To determine the value of d and total dimensional compression, Hertz contact theory can be used. The assumptions made in the Hertz theory are as follows (Johnson, 1985): 1] The surfaces are continuous and non-conforming; 2] $d \ll r$ is the radius of the contacting body; 3] The strains are small; 4] $d \ll r$; 5] Each solid can be considered

as an elastic half-space of $(1/2)\pi$ -inch length of the cylinder, and (b) the surfaces are frictionless.

For the rhizome problem, the first and third conditions are met. Johnson (1985) states that the Hertz theory is valid for some repetitive materials. For contact of solids where each could have surface layers whose elastic properties differ from inner layers, Hertz theory is valid as long as the top layer is large compared with the contact area (d). Also, rhizomes are composed of material similarly to that in some soils, and the Hertz contact theory predicted accurately the values for d (Chikazawa and Asanaka, 1994). Therefore, the use of the Hertz contact theory instead of a large deformation theory to predict contact compression probably would not lead to elastic errors. Finally, although the rhizome is covered with roots which could cause friction between contacting rhizomes, Hertz theory can be used for sphere geometries because friction at the surfaces of two non-conforming bodies brought into normal contact plays a part only if the elastic constants of the two materials are different (Johnson, 1985). Therefore, the Hertz contact theory can be used for sphere geometries, and the following formula for total compression, d , of a rhizome diameter compressed between two other geometries through the mid-points of the contact areas was used:

$$d = \frac{2}{3} \frac{(1+\nu)^{1/2}}{E} \left(\ln \left(\frac{4}{\pi} \frac{r}{d_1} \right) + \ln \left(\frac{4}{\pi} \frac{r}{d_2} \right) - 1 \right) \quad (8.20)$$

where, d_1 and d_2 are the non-contact-widths from Hertz contact theory given by

$$d_c^2 = 4 \pi r r_c \ell^2$$

where ℓ^2 is the composite modulus of the two connecting cylinders or

$$\ell^2 = \frac{L_1 d_1^2}{E_1} + \frac{L_2 d_2^2}{E_2}$$

Materials and Methods.

In this section the procedures used to determine Young's modulus and Poisson's ratio for water hyacinth rhizomes are discussed. These values will be used whenever the total dimensional compression of a rhizome is needed (Equation 4.20).

Rhizomes were collected from Keweenaw Ave. a lake where water hyacinths are not harvested; consequently, the rhizomes were long. Long rhizomes were collected so that long test samples could be cut. Samples were 2 to 3 cm long and 1 to 3 cm in diameter because these dimensions are typically prepared for a compression test (Bridal, 1984). Nodes were left on the rhizomes to correspond to an initial compression condition. After the samples were cut, rhizomes were observed to be rounded and covered with a soft, spongy tissue which probably represented internodal material. The internodal area produces the roots and leaves. The amount of spongy material varied from sample to sample. The samples were subjected to radial compression on an Instron universal testing machine until the rhizome crushed. Loading rates of 10, 50 and 100 mm/min were used to check viscoelastic behavior.

Typical force-deformation behavior of a radially compressed rhizome composite exhibits two distinct stages. During the beginning

of the test, the resin and sponge material flatten with very little resistance. This behavior is non-linear. Thus, there is a linear elastic region where the primary resistance to radial deformation is the viscous beneath the sponge material. Unlike many biological materials, rhizomes do not have a yield-point; although, a definite rupture point was evident.

Young's modulus was estimated from Equation 4.10 with values of deformation and force from the linear portion of the force-deformation curve. Poisson's ratio was calculated by the method of Chaberski and Janinski (1981) and the experimental setup is briefly reviewed below.

Successive photographs of the cross-section of each rhizome sample were taken with a video camera during radial compression. The test ended when the sample crushed. The video was viewed frame by frame and the total lateral extension and axial compression were measured. These measurements were used to calculate Poisson's ratio by the use of Equation 4.10.

Resin and Rhizomes

Poisson's ratio was experimentally determined for two samples because other samples tended to rotate under the flat plate, which, made measurements difficult with video analysis. These two values are 0.48 and 0.45. They are considered typical for a material that approaches that of a liquid (Rohsenow, 1978). The rhizomes are 10 percent water, therefore, these values are expected. Hence, the average value, 0.47, was used in the calculation of Young's modulus.

The Young's modulus ranged from 3.6 to 11 MPa with an average of 6.9 MPa (Table 4.10). Loading rate appeared to have no effect on these values. The magnitude of the modulus was between those for cork and

(18 MPa) and failure load/peak points (8.4 MPa) (Chakravarti and Senapaty 1984, and Bhattacharya, 1988).

Further, the force per length of ribbon at the transition point of the force-deformation curve ranged from 20 to 40 N/cm. As was mentioned earlier, this force corresponds to the force required to flatten the roots against the ribbons. From the data, the roots approximately projected 0.5 cm apart from the ribbons.

4.3.4 Summary

Polman's ratio was determined to be 0.42 and the apparent Young's modulus: 8.8 MPa. The force required to flatten roots against a ribbon is between 20 and 40 Newton per meter of ribbon length. An equation was presented that could predict the compressive forces involved when ribbons contact.

Table 4.2: Experimental values of apparent Young's modulus of water hyacinth ribbons.

Loading velocity cm/min	Young's modulus MPa
10	10.8
20	8.8
30	6.1
50	11.3
60	10.8
70	7.8
100	16.4
120	6.7
150	10.8
Average	8.8
Standard deviation	6.2

CHAPTER 5 MATHEMATICAL MODEL OF EARTH QUAKE SHOCKING TEST

This chapter is concerned with development of a mathematical model of a water liquefaction shaking system. It is based on physical laws and elements of the system and their interconnections ship. Principal system variables are soil horizontal velocity and displacement. Using testing velocity as the input, the model was solved numerically, and calculated values of testing force and soil response were compared to experimental data. Further, in this chapter the physical condition of the system prior to vertical displacement or "roll under" is described.

Generalized Model

To construct an accurate mathematical treatment of components and their interconnections, knowledge of the structure of the system is required. At the beginning of this study, physical components of a water liquefaction test and their interconnections were obscure and ill defined. As the picture of different component interconnections was manipulated until the resulting mathematical model generated accurate input-output relationships for different boundary and input conditions. The resulting conceptual model of a mechanical system that consists of a water liquefaction test of length L , and width B , and a device that actuates them is shown in Figure 5-6. The distance from

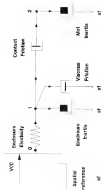


Figure 1.0: Schematic model of a motor force-actuated system with a moving base

Mechanical System

node 1 to node 2 is the length l_0 of the actuator, node 1 represents the rear of a unit, and node 2, its front end.

The actuator device is analogous to the test equipment used in the testing experiments described in Chapter 4.3. For other testing devices, different kinds and arrangement of elements could model the actuators. It is represented by a spring and a mass element. The spring (representing the actuator beam in the testing test apparatus) is connected to a frame of finite mass which surrounds a unit.

The water hydrodynamics are represented by two types of friction elements and a mass element. One of the friction elements is associated with viscous friction of the water hydrodynamics moving through the water. As the system begins to move, motion is imparted first to its rear. It is for that reason that the viscous friction element is positioned at node 1. The other, connected between nodes 1 and 2, is associated with sliding friction of contacting particles.

Initially the system is at rest. At time $t > 0$, the actuator moves at velocity $V_0(t)$. As the act begins to move, it compresses. When compression ends, the entire act moves uniformly at the speed of the actuator device.

Element Descriptions

Mass elements. To represent part of a physical system by an ideal constitutive mass, it is necessary that all of the particles be connected together and that they move with substantially equal or proportional velocities and accelerations (Murray et al., 1971). The constitutive equation for an ideal mass element is derived from Newton's second law and integral form: F_B equals

$$F_{B_j} = B_j \frac{dV_j}{dt} \quad (3.5)$$

where, j denotes mode number

B_j = mass of the structure $= B_0$

B_j = mass of water displaced and plus mass of water accelerated with the m.s. $= B_{w_j}$

V_j = Velocity a water displaced m.s. of mode j with respect to spatial reference

Water mass is added to that of the water displaced m.s. because it gives the additional force required to accelerate the mass of fluid out is action by the acceleration of the plate partly submerged in the water. The additional amount of water mass depends on the shape of the moving object (Garbat and Gross, 1985). Water displaced structures, the principal objects in flow, are similar to circular cylinders. For circular cylinders in two-dimensional flow, the apparent increase in mass due to water equals the mass of fluid displaced by the circular cylinder (Froude) and Tuckers, 1984).

Further along, any mechanical element that undergoes a change in shape when subjected to a force can be characterized by a stiffness element, provided that an algebraic relationship exists between the elongation or compression and the force. The constitutive law used in the testing experiments can be identified as one stiffness element. For a linear constitutive elastic force F_k equals

$$F_k = K X_{k1} \quad (3.1)$$

where K is an elastic constant and X_{k1} equals the constitutive deformation that is given by

$$X_{01} = X_0 - X_1,$$

where X_0 is the displacement of the spring device and X_1 is the displacement of a meter (usually one or more). Also, the rest deflection is defined to be zero, with $X_0 - X_1 = 0$, and no force applied to the container.

For small deflections, δ , of a cantilever beam, K can be calculated as follows:

$$K = \frac{F}{\delta} = \frac{3EI}{L^3} \quad (2-2)$$

Where F = concentrated load at free end of beam.

L = beam length.

E = modulus of elasticity.

I = moment of inertia = $\frac{bh^3}{12}$

b = beam width and

h = beam thickness

For $b = 0.5$ in., $h = 0.175$ in., $E = 0.30 \times 10^{11}$, and K (for steel) = 2.07×10^{11} Pa. K is 3,000 N/m. The experimental spring device and four cantilever beam spring collectively as the theoretical value of K is 12,000 N/m.

Another useful property of the idealized mechanical device is its natural frequency, ω_n , is easily given by

$$\omega_n = \left(\frac{K}{M} \right)^{1/2} \quad (2-3)$$

The period of vibration of a submerged mechanical device depends on the mass of encasing device M_0 and on K of its suspension beams, and is independent of magnitude of oscillations. For an enclosure mass equal to 10.0 kg, and K equal to 12,000 N/m, the system natural frequency is 5 Hz.

Viscous damping. Drag friction is primarily due to the presence of water against chimneys. If, in addition, the enclosure device is equipped with objects that protrude into the water, different types of viscous elements may have to be added to the model. In Chapter 4, the description of drag due to chimneys was found to be a nonlinear function of wet velocity. In the empirical model, wet velocity is the velocity of node 1. Therefore, the steady flow description of the viscous element is

$$C_D = 1000 \, k_p \, k_v \, C_d \, V_1 \, |V_1| \quad (2-6)$$

The absolute value symbol is required to change direction of force with change in direction of velocity. Drag coefficient for steady flow was found to be $C_D = 0.008 \exp(0.000 \frac{V}{1})$ [Equation (2-6)].

Drag in a real, incompressible flow was not described experimentally because several physical components in the system were simultaneously active (e.g. inertia, drag, and contact friction elements), and drag coefficient is not known. What a bluff body starting from rest moves through a fluid, moves and rotates freely and changes with time. Viscous drag is proportional to the rate of increase of energy resulting from the lengthening of these waves [Peters, 1984]. Also, the drag coefficient of a group of spherically shaped particles moving

with acceleration in a turbulent fluid was found to be functions of particle Reynolds number and degree of turbulence, but not of the acceleration (Ritter and Ross, 1968 and Tanioka and Garton, 1984). Therefore, the value of C_D is steady flow and not be used in Equation 5-6 while a net is accelerating, because the testing experiments and theory described in the previous chapter give a precise mathematical description of drag only when the rate of increase of velocity is constant, as also the effects of aquatic acceleration are shown. Therefore, an expression for C_D that is valid for unsteady flow conditions was obtained through model simulating.

Contact Friction. Another friction force arises between plants can be obtained in each other through entanglement of particles, a condition that occurs when the population density is high and when the plants are healthy and have not been attacked by insects or disease. Amount of particle entanglement is measured by connectivity (see Chapter 3). When entangled particles slide against each other during relative motion, friction resists that motion. As a minor exception, net supports, particles slide over each other. A film of liquid exists between these particles, because leaves are wet. If the resulting fluid flow is laminar, the following describes the optimum sliding force F_G^1 :

$$F_G = K_G V_{GL} \quad (5-5)$$

where $V_{GL} = V_1 - V_2$; variables V_1 and V_2 correspond respectively to horizontal velocities of the front and back ends of a net.

Spreads out with respect to a spatial reference, and R_L is the interaction plane friction coefficient, which is proportional to the contact area and the mean viscosity and inversely proportional to the thickness of the film [Chen and Friedrich, 1979]. Parameter R_L can be evaluated through simulation.

Model Formulation

In this section, a set of equations were developed that described a water-lubricated bearing system. First, node equations are written; then, a set of state-variation equations is derived, and finally, a set of output equations is derived from the state variations.

Node equations. The system model has two nodes; therefore, two vector equations describe force interactions at these nodes. Applying Newton's third law, the sum of the forces at these nodes must equal zero. The node equations are

Node	Equation
1	$-F_h + F_{R_L} + F_f + F_a = 0 \quad (6-6)$
2	$-F_a + F_{R_L} = 0 \quad (6-7)$

State-variation equations. A set of state variables describes completely the behavior of a system in response to its inputs. Therefore, knowledge of their values at reference time, t_0 , and the values of the inputs for all $t \geq t_0$ is sufficient for evaluation of state and output variables of the system for all $t \geq t_0$ [Chen and Friedrich, 1979]. One state variable is usually associated with each energy storage element (e.g., inertia and spring elements)—for this study, the state variables are \dot{R}_{ax} , \dot{R}_z , and \dot{R}_y . The former pertains to mean-lower deflection, and variables \dot{R}_z and \dot{R}_y correspond respectively to horizontal velocities of the front and

back ends of a water hydraulic actuator respect to a spatial reference. To derive state variable equations, an expression for the derivative of each state variable is formulated in terms of other state variables, time, and inputs by using model equations and component descriptions. The description used for the rate of change of X_{21} is derived below, and it is in terms of input variable X_1 and state variable X_1 .

$$\frac{dX_{21}}{dt} = \frac{d(X_1 X_{21})}{dt} = X_1 + X_1 \quad (5-14)$$

The state variable equation for X_1 was derived in the following way. First, the time derivative of X_1 was expressed by using the component description of the inertia of the nonlinear device [Equation 5-10]:

$$\frac{dX_1}{dt} = F_{X_1}/M_1 \quad (5-15)$$

Next, another expression for F_{X_1} was found since a state variable equation is in terms of other state variables. It was obtained from model equation 5-8, and substituted into Equation 5-15:

$$\frac{dX_1}{dt} = (F_1 - F_2 - F_{c1})/M_1 \quad (5-16)$$

Finally, the second state variable equation was obtained after appropriate component descriptions were substituted into Equation 5-16:

$$\frac{dX_1}{dt} = [K X_{21} - 1000.0 B_F A_F C_d V_1/P_1] - B_L[V_1 - X_1]/M_1 \quad (5-17)$$

The final state variable equation (Equation 5-10) was derived in a similar fashion:

$$\begin{aligned}\frac{d\bar{Q}_2}{dt} &= F_{B_2}/\bar{Q}_2 - F_{A'}/\bar{Q}_2 \\ &= [K_2 (V_1 - V_2 - 1)]/\bar{Q}_2\end{aligned}\quad (5-10)$$

Calculations. For comparison, towing force, sliding contact force and drag force are output variables of the model of interest, and they are calculated at any time t , from the state variables. Sliding contact and drag force are computed from Equations 5-4 and 5-5. Net compression for a set of original length, L , is computed from:

$$\text{Net compression} = L - (X_2 - X_1) \quad (5-11)$$

where X_1 and X_2 are calculated after the following equations are integrated:

$$\frac{dX_1}{dt} = V_1 \quad (5-12)$$

$$\frac{dX_2}{dt} = V_2 \quad (5-13)$$

Towing force is calculated:

$$\text{Towing force} = K (X_{01} - X_1) \quad (5-14)$$

Method of Solution and Parameter Estimation

Equations 5-8, 5-11, and 5-12 are a system of coupled first-order differential equations, one of which is nonlinear. They represent a water hydraulic towing system. The system was solved in discrete steps to reduce the towing force study mechanism of "full order" and net compression, and determine parameters C_D of steady flow conditions and Q_0 .

To solve the equations, initial conditions, estimates of parameters, \bar{F}_0 as a function of time, and numerical integration were needed. System variables and outputs were evaluated numerically because one of the equations was nonlinear and two parameters were unknown. The initial conditions most needed to compute inputs and some output variables, at t_0 , \bar{F}_0 and \bar{V}_0 , were always set at zero. Displacement of the spring element was zero ($X_0 = X_c = 0$) and X_0 equal to L . Further, at $t \geq t_0$, velocity \bar{V}_0 was a function of time, and system variables and outputs were evaluated numerically until the system reached steady state or until the derivatives of the state variables were zero.

The fourth-order Runge-Kutta formula was used to solve the equations in a "problem solver" program written in Turbo-Pascal (Appendix A). The program utilizes a published version of Runge-Kutta (Press et al., 1988). The step size, based on accuracy, was 0.02 seconds for an integration interval of 10 seconds.

To estimate the parameters \bar{R}_L and \bar{C}_D , the following scheme was adopted. The program accepted estimates for parameters \bar{R}_L and \bar{C}_D , solved the differential equations, and returned values of testing force and net suspension as specified times. Output values were compared to experimental values that were generated during the steady part of the testing experiments discussed in Chapter 4. This procedure for parameter estimation was repeated until correlation between calculated and experimental data was better than 80%. Testing forces as main data shown described in testing experiments 4 through 7 (Table 4.2) was calculated. Plots in the main chapter were made to help

Initial estimates of parameters C_d and R_L were based on some prior information. For instance, the drag coefficient in steady flow conditions was assumed to equal the experimental value for steady flow conditions at large α . The other parameter, R_L , was thought to be constant because physical factors that could affect it do not vary appreciably in a water hydraulic mt. Friction coefficient R_L is known to be proportional to contact area and viscosity and inversely proportional to film thickness. In water hydraulics, film thickness and viscosity do not vary and contact area probably varies slightly as α increases.

Results of Parameter Estimation and Model Results

Parameter values. Correlation coefficients between model and experimental data for six different conditions ranged from 0.88 to 0.98 (Table 5.8). The lower coefficients were associated with experiments and calculated using force computed at $\tau = 0$. This was partly due to uncertainty of the exact time a experiment started (i.e., the beginning of an experiment was defined to be the time of the first nonzero data point recorded by the data logger). Also, two few data points were obtained because experimental data were recorded every second (i.e., large sampling time).

Expressions describing R_L and C_d selected on the basis of these correlations (Table 5.8) were found to depend on plant size and other physical characteristics of a mt. Further, drag coefficient was found to vary with R_L until steady conditions were reached and it is described by the following equation²:

$$\mathcal{C}_d = 0.008 \exp\left(0.008 \frac{U}{L}\right) \quad (5.26)$$

where

$$L = L_q + L_0 \quad \text{for } V_n < V_c$$

$$L = L_0 \quad \text{for } V_n = V_c$$

where L_q is a constant. This expression supports the assumption that a net moves at constant speed when both the front and back ends move at the same velocity, and the steady drag coefficient of a net changes to its steady flow value.

Table 5.2. Correlation coefficient between model and experimental towing force

Reynolds $L \times U$	Towing velocity m/s	Number of data points	r^2
1.02 m \times 1.00 m	0.05	6	0.99
1.02 m \times 1.00 m	0.14	6	0.99
1.02 m \times 1.00 m	0.21	6	0.99
0.60 m \times 1.00 m	0.05	6	0.99
1.02 m \times 1.30 m	0.05	6	0.99
0.60 m \times 1.00 m	0.05	6	0.97

The constant L_0 was necessary because without it \mathcal{C}_d would not have a finite value at small values of L_q . Constant L_0 was determined to be 0.5 for three of the experiments modelled and is equalled 0.36 for a net that was comprised of large plastic and large shrimps with long rostra (Table 5.1). The experimental number three represents the towing experiment modelled. This constant determines the value of the drag coefficient just as a net starts from rest. As the value of L_q decreases in different types of nets, the drag

coefficients increases. Therefore, it is conceivable that very long roots and large volumes would affect it. In addition, R_p was not affected by root geometry or towing velocity.

Table 5.1 Values of R_L and R_p of the nets modeled.

R_L	R_p	Experiment modeled	average plant length, cm
50.0	0.50	6	25.0
140.0	0.48	4	50.0
170.0	0.50	7	75.0
140.0	0.56	8	90.0

The values of R_L determined for each net modeled is presented in Table 5.1. It was affected by the type and condition of the plants that comprised a net, but it was unaffected by net geometry or towing speed. The net with the lowest value of R_L (50) was comprised of very small plants. The friction factor of nets of larger plants varied from 140 to 170. This range of values could be due to sensitivity. Small plants were connected primarily by nodules, and larger plants were connected primarily by overlapping leaves.

Effect of towing velocity Nets were accelerated experimentally in one of two ways. For towing velocities less than 0.25 m/s, the accelerometer provided a step velocity input, and for towing velocities ≥ 0.25 m/s, the nets were accelerated incrementally to offset possible detrimental inertial effects on the testing equipment.

The following describes model response to a single step velocity input. When it was first tested, testing force increased very quickly and then decreased gradually to a steady value. The maximum value occurred independently of test area at approximately 0.1 s , because inertia due to the mass of the enclosure is the principal active component in the system up to that time which was expected because the natural frequency of the spring - massless system was 5 Hz . As speed increased and test length increased, it took longer for the force to decrease to a steady value (Figures 5.1 and 5.2). For a sample size 2.44 m long by 1.33 m wide, time to reach steady conditions ranged from 2 s for a velocity of 0.2 m/s to 10 s for a velocity of 0.8 m/s .

Sliding friction (Equation 5.4) and viscous friction (Equation 5.5) are responsible for damping (Figure 5.3). Viscous friction increases as the square of velocity; so, as testing velocity increases, it quickly becomes the principal active component.

Net compression as measured at steady state, increased linearly with testing velocity. When V_L equaled 10 , compression of a 2.44 m long \times 1.33 m wide π net increased with testing velocity according to this expression:

$$\text{net compression} = 1.43 + V_L$$

To allow the effects of inertia of the enclosure, mass was minimized incrementally whenever the desired final testing velocity was greater than 0.25 m/s . Increments varied according to the discretion of the testing operator. Velocity inputs to the problem solver were changed incrementally with time in an attempt to obtain these experiments. Two conditions were done: one involved a bed with very small plants, the other involved medium-sized plants typical of a

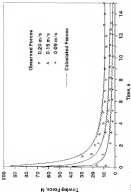


Figure 5-3 Effect of towing velocity on a 2.64 m long \times 1.17 m wide net that was comprised of uniform-sized plastic averaging 80 μ m in length. Flowsheeting due to commercial installation related to T_{12}

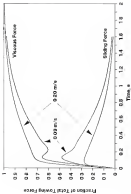


Figure 5.3 effect of towing velocity and drag on a cat. The cat measured 2.64 m in length and 1.03 m in width. It was configured at midline-tilted planes averaging 44.6 deg in length.

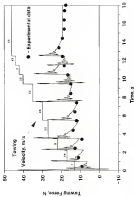


Figure 3-4. Towing force for a 2.44 x 3.66 x 3.33 m wide net accelerated incrementally. The net was comprised of twill fabric stretching 0.50 m in length.

water treatment system. The setup is presented in Figure 5.4. Experimental data showed that the model accurately predicted the increase in testing force with testing velocity. The model predicted that every time velocity was incremented, there would be a peak in the testing force due to the inertia of the mechanism device, and that the friction forces generated by the plastic were not great enough to immediately affect the oscillatory response of the combined spring-mass components. However, these predicted peaks in the testing force were not evident in the experimental data. This could have been due to the sparseness of experimental data in these regions. Also, the model used a step velocity increments and in practice, step inputs are nearly impossible to obtain.

On the other hand, a set comprised of larger plastic ball friction forces large enough to dampen quickly the combined response of spring-mass components (Figure 5.5). Peak force due to inertia of the mechanism was predicted by the model, but was not evident in the experimental data, although, the model accurately predicted the general increase in testing force. This discrepancy probably was related to the practical impossibility of a step velocity input.

The advantage of reducing inertial effects through a slow increase in testing velocity is that the maximum testing force can be reduced by a factor of four (Figures 5.6 and 5.6). The "problem solver" program can be used to model the response of the system to other kinds of velocity inputs. Different ramp velocity inputs were modeled (Figure 5.7), and results indicate that inertial forces could be negligible for small accelerations.

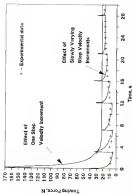


Figure 3-3: Buffership 30 Towing force on a 2.44 m long ± 1.37 m wide catamaran towed by a 0.25 m/s. Experimental data was obtained from a test where the catamaran comprised of nothing about planes extending 0.25 m in length.

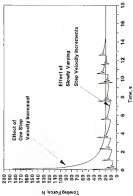


Figure 3.4: Differences in towing forces on a net due to the way it was accelerated in Gull net. The velocity step and velocity steps are illustrated as shown in Figure 3.4.

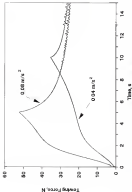


Figure 5.1 The curve represents forces measured on a 1.44 m long, a 1.44 m wide and comprised of three planks that was given a 1 mm velocity jump to initiate a third velocity of 0.4 m/s in 3 s. Initial curve represents request due to a slower acceleration where final peaking reflects one reached in 10 s.

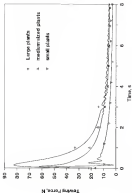


Figure 3.4 Effect of plant size on rooting time. The curves were 2.0 s long for 1.0 s to make the curves relevant and 0.1 s to 0.2 s.

Effect of nut characteristics: The effect plate size has an overriding factor is evident in Figure 5.8. Small plates (population density, 3000 m^{-2}) have small rhizomes, and, although the total number of plants in a mat is large, the total projected area is close to small compared to the expected total area from a mat of larger plants. Therefore, as depicted in Figure 5.8, the rhizome force at low waving velocities on small rhizomes was not large enough to dampen quickly the oscillatory response of the combined spring-mass components. In contrast, oscillatory response was not evident in mats comprised of large plants.

Also, initial oscillatory response of spring-mass components takes longer to dampen [Figure 5.9] if mats are small. As the width of the mat decreased, sliding force contributed a greater proportion of the total force on the spring, and mat compression (X) decreased [Figures 5.10 and 5.11; Table 5.2]. Compression of a 1.00 m long by 0.40 m wide mat was approximately half the compression of a 1.00 m long by 1.00 m wide mat. On the other hand, as the initial length of the mat decreased, mat compression (X) increased slightly.

Table 5.2. Theoretical effects of mat size on compression. The mats were tested at approximately 0.35 m/s , and were comprised of random-sized plants averaging 61.9 cm in length.

Sample Size	Compression	Initial Speed Mass Density
L X W	X	kg/m^2
1.00 m x 0.40 m	1.36	21.3
1.00 m x 1.00 m	2.76	39.0
0.40 m x 1.00 m	3.26	39.0

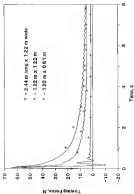


Figure 3.8 Tearing force on different sizes of cable tested at 0.15 m/s. Data were captured at video-estimated frame frequency 60.0 to 120 Hz.

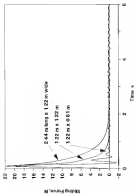


FIGURE 3.10 Curves: friction force due to particle sliding over rods after no rollers, sizes of rods used at 0.25 m/s, rods were composed of stainless-steel plants averaging 60.0 cm in length

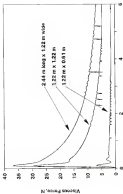


Figure 5.13 Wheaton Power (W) vs. different plant sizes (h) used at 0.15 m/s. Data were computed at medium-sized plants averaging 0.2 m in length.

Physical description of "roll under". Buoyancy acts through the centroid of a displaced volume, and this centroid changes position if the body tips. While a mine hydrofoil is vertical due to support by other plants or by flume, its centroid of displaced volume is positioned near the center of its chord, and its center of gravity lies above the centroid of displaced volume. These conditions make mine hydrofoils unstable vertically (Garhart and Green, 1965). Plants submerge if enough force is applied to overcome the buoyant force or if the plants are tipped so that their metacenter lies below their centers of gravity. Metacenter is defined as the intersection of the buoyant force and extension of the volume of the object.

The buoyant force of a net was described in chapter 3. Buoyant force per unit net area depends on water column density and on the physical condition of the plants. If plants are well connected, force above the normal buoyant force is required to submerge a net due to the support structure of a net. Nets used in this study covered the water at velocities exceeding 0.66 m/s, although, one net was stable up to 1 m/s. This net was prevented from compression, and its leading edge was not free to rotate. In general, the velocity when "roll under" occurred ranged between 0.4 and 0.6 m/s. Calculations indicated that mine used in this study required from 100 to 300 Pa to submerge then $(\rho \cdot g \cdot h = 3 \cdot 10^2 \text{ and required over } 1000 \text{ N to submerge it})$.

Observations indicated that mine comprised of different sized plants and connections became unstable at different values of compression. For instance, a net consisting of small plants became unstable when compression was approximately 30 N, and mine consisting of larger plants became unstable when compression was approximately

100. These compression data were independent of sample size if the test was comprised of plants of the same size and crown density.

One of the assumptions made in the development of the model was that a net compresses until the rhizomes touch (Chapter 4). This implies compression of approximately 100%. After observing that soils consistently "fail under" at lower compression, the model was changed to allow for the formation of bridges in a soil [i.e. enough rhizomes are in contact within the soil to prevent further compression]. To determine the spring response when rhizomes are in contact, another element was added to the conceptual model representing the elastic nature of rhizomes in contact. It is described as follows:

$$\text{Elastic constant of rhizomes} = K_p [1 + (X_1 - X_2) - R_p]$$

where R_p equals the maximum observed compression of a given soil, and $K_p = 10 \text{ mN}^{1/3} \text{ Pa}$. The elastic constant K_p is the elastic modulus of rhizomes calculated in Chapter 4. This element was positioned in parallel with the contact friction element. With the addition of this element, Equations 5-21 and 5-22 changed to:

$$\begin{aligned} \frac{dX_1}{dt} = & [(X_1 - 100) K_p / (X_1 - R_p) V_1 (V_1 - R_p) \\ & - K_p [1 + (X_1 - X_2) - R_p] V_2 / R_p] \end{aligned} \quad (5-27)$$

$$\frac{dX_2}{dt} = [R_p (V_1 - V_2) - K_p [1 + (X_1 - X_2) - R_p] R_p] / R_p \quad (5-28)$$

These equations were solved by the "position solver" program. Results showed that when compression equaled R_p , cooling forces per

unit nail area exceeded 200 Pa. This force was sufficient to submerge nails provided that the plates contact at an angle and a vertical elastic contact force is produced. This is illustrated in Figure 5.4b. Observations showed that plates were tipped and touching at "roll under". Model output beyond "roll under" were considered meaningless because the program solves only one dimensional problems. Furthermore, calculated loading velocity at "roll under" were within 10% of the experimental values.

Summary

-Nails of various lengths were driven within a rectangular enclosure were modeled. The model consisted of one inertial element, one for the nail and one for the enclosure, a constant friction element, a viscous friction element, an elastic element for the enclosure, and an elastic element for release contact.

-None of the parameters required to solve the model were obtained from comparison of experimental and computer simulation data.

- The model accurately predicted loading force and compression for different sizes and types of nails and velocity inputs.

- It failed to predict when a nail becomes unstable was prevented

CHAPTER VI NET COMPACTION MODEL

The model of a water hyacinth net presented in Chapter 5 was used to determine the response of a net constrained at one end to compression from the opposite end. Calculated compression force was compared with experimental data. This information can be used to determine the capacity of spring devices and to design more efficient removal devices.

Conceptual Model

The conceptual model of a water hyacinth net of length L , net width B under a compressive force is pictured in Figure 6-1. In this model, the same components developed in Chapter 5 are used to represent the water hyacinth net. The compressive force represents the test equipment used to compact mats, so data obtained from experiments could be used to validate the net model. The net was allocated between two bars, a fixed pusher bar and a fixed rear bar. The front bar pushing the net was considered rigid; therefore, only its mass is represented. The back bar--on the other hand--has a elastic contact always attached to it, which represents interaction between obstacles and the rear bar.

Initially, the system is at rest. The distance from node 0 to node 1 is the rest length of the net. As the front bar moves to



Mechanical System Compression Frame

Figure 8.9: Diagrammatic model of water level/pressure subjected to compression.

velocity $V_0 = 1$ compresses the net against the back bar, and the net stretches. Compression continues until the plates "roll under."

Mathematical Formulation.

The element descriptions were given in Chapter II. Newton's third law applied at node 1 produces the following equations:

$$F_{R_1} - F_s + F_{R_2} = 0 \quad (3-2)$$

where

$$F_{R_1} = R_1 \frac{dX_1}{dt} \quad (3-3)$$

$$F_s = R_s (V_0 - V_1) \quad (3-4)$$

and

$$R_2 = R_0 (X_1 - 1) \quad (3-5)$$

Force R_1 is the elastic contact force of plates against the front bar, and it equals zero when $R_1 = 1$. Rest length of the net is 1, and R_s is deformation of spring at node 1. Elastic constant $R_0 = 2 \times 10^7$ N/m, which equals approximately the inverse of the compressive modulus of elasticity of the two connecting bodies (rubber and steel) over half initiated by net width, W . The state variable equations are

$$\frac{dX_1}{dt} = V_1 \quad (3-6)$$

$$\frac{dV_1}{dt} = [-R_s (V_0 - V_1) - R_0 (X_1 - 1)] / M_1 \quad (3-7)$$

The output equations were chosen to describe net compensation and total respiratory losses. Compensation losses are calculated on mode 5 from input variables and computed state variables for time $t \geq t_0$.

Compensation Form: =

$$= K_{\frac{d-3}{d-1}} \frac{d-3}{d-1} + 1000 \cdot K_p \cdot K_r \cdot C_d \cdot K_d^2 + K_L (T_0 - T_2) \quad (6.6)$$

where

$$\frac{d-3}{d-1} = \text{constant for a step velocity input}$$

Net respiration was calculated on the basis of the change in specific area, because specific area compensates for differences in initial weed standing densities of sows. Specific area A_s is defined by:

$$A_s = A_1 / R_p \quad (6.7)$$

where R_p equals the mass of the plants within the frame and A_1 equals the area within the compensative frame at time t_1 :

$$A_1 = (A - A_0) \cdot T$$

where A_0 was obtained after the integration of $V_{a0} = \frac{d-3}{d-1} K_d$

Method of Solution and Parameter Estimation.

Equations 4.5 and 4.6 represent a system of coupled third-order differential equations. Together with the output equations, they represent composition of a state equation set. The equations were solved to determine the behavior of a net subjected to different compensation velocities, mechanisms of "soil water", and parameters K_p ,

and Q_0 of unsteady flow conditions. The latter were studied because their descriptions were not presented equal to those for the first type.

To facilitate the solution process and to solve the system equations, the fourth-order Runge-Kutta formula was used, and a program whose routine was written that calculated values of net compression and compressive force every 0.001 s [Appendix B]. Initial conditions were $V_1 = 0$, $R_1 = 0$, and $Q_1 = 0$. Input velocity was a function of time and it was specified to be a ramp in the first 0.2 s and a constant after $t = 0.2$ s. Ramp descriptions were

Type 1 Acceleration:

$$V(t) = \frac{V_0}{0.2} t \quad \text{for } t \leq 0.2 \text{ s} \quad \text{and}$$

$$V(t) = V_0 \quad \text{for } t > 0.2 \text{ s}$$

Type 2 Acceleration:

$$V(t) = \frac{V_0}{0.5} t \quad \text{for } t \leq 0.5 \text{ s} \quad \text{and}$$

$$V(t) = V_0 \quad \text{for } t > 0.5 \text{ s}$$

where V_0 was the steady compression velocity. These velocity inputs were chosen because they modeled accurately the experimental compression velocity. For the experiment, however, type 2 acceleration was used to attempt to produce a better correlation between experimental and calculated compressive force.

Parameters R_L and C_D for towing were obtained in Chapter 5; however, the boundary conditions of a tunnel are different from those in competition. In competition, mice are driven in straight lanes; hence, aerodynamic drag is negligible. Therefore, initial friction coefficients R_L should be low. Also, drag coefficient of a net during actively towing need not be the same as for competition. Therefore, parameters C_D and R_L were estimated for competition. The "probable values" accepted estimates for these parameters, solved the equations, and returned values of towing force and net compression as functions of time. The compressive force was compared with experimental values. This procedure for parameter estimation was repeated until acceptable correlation ($r^2 > 0.8$) between calculated and experimental data of mice for different plate size and different competition velocity was achieved.

Initial estimations of parameters C_D and R_L were based on some prior information. Initially, R_L was held constant since it was found to be a constant in Chapter 5 and C_D was assumed to vary as a net compressed. Because drag coefficient for steady flow conditions drag coefficient was determined to be a function of net geometry [Equation 4.18] and because the dominant characteristics of a net in competition is reduced net length, C_D was initially varied according to a variation of Equation 4.18:

$$C_D = 0.001 \exp(0.001 \frac{L}{L_0}) \quad (8.4)$$

Experimental Compression Tests

A test rig was designed and constructed for this investigation. The 3 m by 3 m compression frame (Figure 8-1) consisted of a roller moving along a pair of tubular rails and a smaller fixed roller. Rollers on 20 m casters extended 30 cm before the roller tube and 30 cm above. The moving roller weighed 22.3 kg, and it was driven by chains on each rail which, in turn, were driven by a hydraulic motor or a modified automotive engine. With these devices four different constant compression velocities could be obtained: approximately 0.05 m/s, 0.10 m/s, 0.50 m/s and 0.8 m/s.

Force and velocity were transmitted from the chain to the roller through a roller support carriage equipped with load cells to measure force and position linkages. Signals from the load cells and from position sensing potentiometer were recorded on a Campbell Scientific DAT data logger equipped with "Basic mode" software which allowed a sampling interval of 0.01 s.

Tests were conducted on plants at the following locations: IFAR field laboratory, Beltsville, FI; USDA Forest Research Unit, OR; and Walt Disney World theme plant, Kissimmee, FI. After the test was completed (Figure 8-3), seven plants were characterized by shoot height, root length, stem, petiole and number of petioles and total length. All plants inside of the frame were weighed, and the weight was divided by the area of the frame in order to determine standing area density. Biomass length and population density were calculated using the equations presented in Chapter 3. Root frequency and connectivity were measured in some experiments, and estimated for others.

Figure 4-1 Conversion System was equipped with a sliding bar and foot
which locked at both ends of the bar. As the bar moved
toward the flag and bar, the end expanded.



Figure 4.2 A water hyacinth net was being constructed



Compression tests were performed on 15 different mats and characteristics of five of these mats are presented in Table 6-3. Every mat was expected to test the model under different conditions of compression velocity, consistency, strand density, plant size and orientation, and to test the equipment. These five mats were sufficient for parameter estimation.

Experiment	Table 6-3 Characteristics of mats compressed				
	P_p	σ_p	A_p	Total length, m	Observation
		kg	m^2		
1	1500	500.0	0.005	0.40	well compressed
2	1500	150.1	0.0008	0.30	
3	1800	80.0	0.0008	0.20	open spaces
4	2000	175.0	0.0008	0.27	
5	2000	150.0	0.0008	0.20	richly

Before a compression experiment started, a grid of nylon cord was placed on top of the mat. The grid was used to determine when "roll under" occurred because it begins when grid lines bend closely together. All experiments were videotaped. After the tapes were viewed, the position of the moving rule at "roll under" was recorded.

Results and Discussion

Experiment Observations Reported observations pertain to force compression behavior, "roll under" and blowability. Compression experiments indicated that as compression velocity increased, the relationship between compression force and compression changed. At low velocity (1 to 25 m/s) the relation between compression force and specific area was approximately linear up to "roll under". At higher velocity (1 to 25 m/s) the relation was obviously nonlinear. The

position within a unit where instability or "roll under" occurred appeared to be the most common area of the unit, and therefore, it occurred at different locations within different units. Generally if plants were well connected, grid lines up to the start of "roll under" moved at the same speed. In contrast, towing experiments showed that motion was imparted first to the rear of a unit than the unit started from rest.

Drag Coefficient. Parameters C_d and R_L varied as a unit responded. C_d decreased with increasing compression velocity and connectivity. The drag coefficient followed the relation given by Equation 8.8, which was further evidence supporting the unit drag coefficient developed in Chapter 4.3.

On the other hand, R_L was found not to be a constant as it was in the towing model, but to increase with displacement of the pusher bar. Friction coefficients vary proportionally with contact area (Olson and Frederick 1974). Since water hyacinth contact area increased with pusher bar displacement and plant population, the following equation was deduced:

$$R_L = R_L^0 R_p X_n \quad (8.13)$$

where, R_L^0 is a constant. After this equation was substituted into the model, calculated and experimental compression forces agreed (Table 8.1). Correlation coefficients ranged from 0.98 to 0.99. The one experiment where agreement was not strong ($r^2 = 0.84$) will be discussed later.

Table 5-1 Contact friction coefficient and other variables used in the simulations of soil compaction

Exp. #	Compaction velocity m/s	$\frac{R_{\text{c}}}{R_{\text{p}}}$	Ball-Binder position mm compaction	Binder bar acceleration cm/s^2	Correlation between experiment and model r^2
4	0.050	117%	31	1	0.98
3	0.30	726%	30	1	0.91
5	0.10	954%	21	1	0.90
6	0.30	350%	21	0	0.88
1	0.40	103%	43	1	0.89
2	0.40	33	14	1	0.81

¹ See text for explanation

Friction coefficient, $\frac{R_{\text{c}}}{R_{\text{p}}}$, decreased with compaction velocity and connectivity (Table 5-1). Plots in experiments 3 and 5 were of low connectivity and plots in experiments 1, 2 and 4 were highly and highly connected. Friction coefficients of different soils compared at the same velocity are always significantly lower in soils of lower connectivity. Friction coefficients of well connected soils decreased linearly with compaction velocity, and the regression relation follows ($r^2 = 0.98$)

$$\frac{R_{\text{c}}}{R_{\text{p}}} = 12445 - 3433 \cdot V_{\text{c}}$$

The few data points were available to obtain a regression equation for soils of low connectivity. However, $\frac{R_{\text{c}}}{R_{\text{p}}}$ of these soils ranged from 33% to 103% of those of highly connected soils. Also, for compaction

velocity exceeding 0.25 m/s, R_L approached the values obtained from cooling experiments.

An explanation for R_L as a function of velocity can be found in Chapter 5, but compression increased linearly with cooling velocity until "roll under". This implies that a net reaches a unique compression and "rolls under" without need for a new maximum if net velocity is high enough. During compression experiments, however, "roll under" in a net is achieved at lower compression velocity by restricting its roll forcing it to compress. Therefore, more friction force is required at low compression velocities to compress a net until "roll under", and it is reflected in a larger friction coefficient.

Roller behavior and roll-up of "roll under". At low compression velocity, the fraction of total compressive force due to contact friction was great; at higher velocities, it was small. Samples of experimental data showed that at low compression velocity the relation between compressive force and specific area [Figures 6.3 and 6.4] was linear. Calculated drag indicated that it does not dominate at velocity below 0.25 m/s, but it becomes dominant very quickly above 0.25 m/s. At the same time, contact force becomes less important [Figures 6.5 - 6.6].

In general, compressive force increases as compression velocity increases [Figure 6.7]. The model could not accurately predict compressive force in a net that contacted initially at very open spaces [Figure 6.8]. Connectivity is considered to be zero in these cases. Furthermore, bursts of the moving bar caused a rapid increase in compressive forces at the beginning of compression [Figures 6.9 and

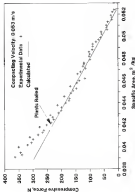


Figure 5.1 At low compacting velocity and before phase "wall order", compressive force remains primarily at contact between. Pore size reduction increases linearly as a new compact

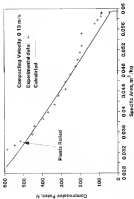


Figure 4. In this reflector, the principal force was due to contact between pores. Pore-pore coefficients decrease as velocity increases.

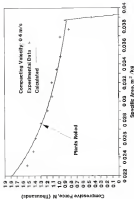
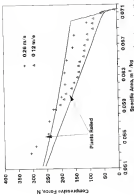


Figure 6.3 At 0.4 m/s velocity, compressive force is primarily due to drag. The increase in force at 0.86 m²/kg specific area is due to effects of the compression bar.



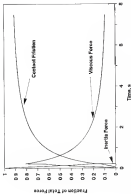


Figure 6.7 The relative values of inertial (solid line), viscous (dashed line) and contact (dotted line) forces over time during the compression of a particle. The relative values of the forces are shown for a particle of diameter 0.1 mm, density 1000 kg/m³, and a compression rate of 0.1 m/s.

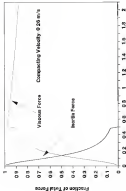


Figure 1: 1. When compaction velocity was higher than 0.25 m/s, compaction force was due primarily to viscous friction (dash). Inertia was due to the compression bar. Contact force was not a major component.

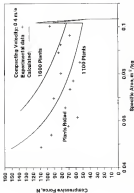


Figure 4.9 The net, composed of five plants and many open spaces. The solid area are mixture with dense conditions. Curves represent low compact conditions, one with 1000 plants and the other with 10000 plants

6.3], because the mass element representing the mass of the compressive bar was the principal stress component of the system.

Compressive [X] or "roll under" varied according to plant population and plant length. Generally, it decreased as population increased and average plant length decreased. Root length was not a factor since all plants tested had short roots. "Roll under" occurred between 50 and 600 compression, which was higher than the values in spring tests. "Roll under" occurred at the front end of a net during spring tests but inside the net during compression tests. Therefore plants were naturally supportive in compression, making it more difficult for them to upset the value. Compressive force on a net of large plants was large enough to submerge the net, provided the plants were rigid to create the required vertical force component. Except with nets of large plants, "roll under" was usually accompanied by a large increase in compressive force.

Summary

A mathematical model was developed to predict the response of a water hyacinth net to forced compression. Initially unknown parameters, C_2 and R_1 were estimated through comparison of calculated and experimental compressive forces. Friction coefficient varied with compression velocity, compression, and consistently. Drag coefficients varied with the change in length of a net.

The model for net compression accurately predicted compressive forces. "Roll under" can be predicted on a basis of compression or buoyancy.

Compressive force was dominated by drag at velocity exceeding 0.30 m/s and by tension force at velocity below 0.10 m/s.

- Compressive force increased with compaction velocity
- Compaction (K) at "roll under" ranged from 20 to 400, and it decreased with increasing plate population and decreasing average plate length. It was not affected by compaction velocity

CHAPTER VII CONCLUSIONS AND RECOMMENDATIONS FOR FURTHER RESEARCH

Water hyacinths were described quantitatively in terms of plant mass, aerial density, root frequency, root number, and rhizome length. These physical characteristics were parameters in hydro-mechanical models describing (1) root bending and (2) root lateral expansion. Critical physical properties of integrating root and stem modules are average plant length, standing density, and average plant mass.

To develop any hydro-mechanical model a relation between drag and velocity is required. Newton's drag law was used to describe this relationship for a root. In this case, area in the direction of flow is projected area of the total number of rhizomes. Drag coefficient is independent of Reynolds number, and it is related nonlinearly to root geometry. A higher drag coefficient is associated with higher width-to-length ratio.

A root expands when it is bent or forced against a barrier. Generally, for expansion between 20 and 60% (Computed $(\text{length}/100)$ rhizomes expand together, and elastic forces are generated. By the use of large deformation theory, elastic properties of rhizomes were

described. Polunin's rolls and Young's modulus were determined, respectively, to be 0.47 and 0.8 MPa.

Conceptual models were developed to describe machine wear dynamics interactions for two different harvesting operations: rectangular-cut taring and forced compression. Mass and elastic elements of test machinery, particle contact friction elements, mass element of a set, set viscous element and viscous elastic compressive element were components of these machine wear hypothesis systems. Their descriptions and interrelationships were based on experimental observations and physical and biomechanical properties. The equations derived from the conceptual model were solved numerically using a Runge-Kutta integration method to study system behavior in response to velocity input. Output variables such as taring or compression force and set compression were predicted over time with good correlation with experimental data. The feasibility or "roll under" was found to occur at a specific set compression.

In a test for the design of better harvesters, the model could be used to determine:

- Optimal taring and compression conditions for different harvesting schemes and
- Effect of different acceleration patterns
- Power and energy requirements of different designs

The design of equipment, such as sensors, rollers, removal devices, and restraining barriers, could be based on the following biomechanical information from this research:

- Slowing, accelerating a set and moving machine parts to reach a constant velocity uses energy. Because the highest forces in

the systems studied were due to Martin, plates should be kept moving at constant velocity

-Drag force is reduced by using long narrow pins...

-"Bell under" is prevented by maintaining appropriate plate density and operating speed.

-Changing a net to decrease its drag coefficient could save towing costs

Additional research should be done for large scale verification and application of the model. Research into parameters concerning and particle friction coefficient could be investigated to determine an empirical relation between the two parameters. Dependency of this coefficient on other plate characteristics could be clarified, too. Also, because C_d was determined only for rectangular pins, drag coefficient could be determined for other pin geometries. Furthermore, development of the problem solver * into a software program with input and output screens is needed for user convenience.

Finally, the movement only of a net and moving machine parts was investigated. Study of water flow under a moving net, which is a very complicated problem, could describe net movement due to negative accelerations.

Appendix A Problem Solving Program

Listing of MODULA-2, page 1

```

PROCEDURE d1000(input,output);
(* driver for routine D1000 *)
uses int;
CONST
  nmax=4;
  nmax2=2*nmax;
TYPE
  d1000 = array(1..nmax) OF float;
VAR
  i,j, count, index : integer;
  a1,a2,Vol,El, Ep, W, Av, T1000,convmax,convmax2,convmax3
    : array(1..nmax) OF float;
  d1000 : array(1..nmax) OF float;
  d10002 : array(1..nmax) OF float;
  d10003 : array(1..nmax) OF float;
  volume : float;
  x0 : array(1..2000) OF float;
  y0 : array(1..2000) OF float;
  x : array(1..nmax) OF float;
  y : array(1..nmax) OF float;
  fact : real;
(*-- MODULA-2 --*)
BEGIN
  TYPE
    d1000 = array(1..nmax) OF float;
  FUNCTION mg1(x : float) : float;
  BEGIN
    mg1 := x;
  END;
  PROCEDURE glines(VAR linfo : int; filename : string);
  BEGIN
    assign(linfo, filename);
    reset(linfo);
  END;
  PROCEDURE deriv(x : float) : float;
  VAR dydx : float;
  (* The arrays x and dydx must carry the dimension given to
  them in the calling routine --*)
  BEGIN
    (dimension(x[1]), convmax:=x[1]); exit;
    IF ( x >= 1.0E+5 ) AND ( x < 3.0E+5 ) THEN begin
      vol:=0.0;
      convmax := x[1]-dimension1;
      dimension2:=convmax;
      exit;
    END;
    IF ( x <= 5E4 ) AND ( x < 3.0E+5 ) THEN begin
      vol:=0.000;
      convmax:=x[1]-dimension2;
      dimension3:=convmax;
      exit;
    END;
    IF ( x <= 5E4 ) AND ( x < 3.0E+5 ) THEN begin

```

Listing of NCNLS2.F90, page 3

```

real :: d0
compute my[1]=distance(
distance=compute
and
if ( x<0.5 h) and ( y < 7.5 h) then begin
real :: d00
compute my[1]=distance(
distance=compute
and
if ( x<0.7 h) and ( y < 9.5 h) then begin
real :: d00
compute my[1]=distance(
distance=compute
and
if ( x >=0.95 h) and ( y < 10.5 h) then begin
real :: d00
compute my[1]=distance(
distance=compute
and
if ( x >=0.5 h) and ( y < 11.5 h) then begin
real :: d0
compute my[1]=distance(
distance=compute
and
if ( x >=11.5 h) and ( y < 12.5 h) then begin
real :: d00
compute my[1]=distance(
distance=compute
and )
constant :: c2000000
if ((length-[x[1]-y[1]]) > 0.0001) then constant2 =d00
else constant2=0.0
if (c1=0)
constant =0.000
distance =d[distance]/constant,
if (([x[1]-y[1]]< 0.000)
and ([x[1] >= 0] and ([y[1] >= x[1]] or
[ constant <length ]
then distance2=length else
distance1=compute+constant
write (dmyout,*) d0/dmyout/0.000write(d0/d0/0.0001)write(x[1])write(y[1])
dmy[1] =my[1]
dmy[2] = 1.0/75.0 dmy=constant+([x[1]-distance])
dmy=[x[1]-y[1]]-or (c1=constant+length+([x[1]-y[1]]< 0.000))
dmy[3] =my[1]
dmy[4] = 1.0/75.0+([x[1]-y[1]]+constant+length+([x[1]-y[1]]< 0.000))
END
CNC: END F90
2 PROGRAM main;glarray= glarray, a integer, with real double float
3 real prec= glarray
4 [a Program using routine NCNLS2 must provide a
5 PROGRAM, here it is end] : glarray= 768 dmy=glarray].

```


**Appendix B:
Protein Solubility Program
For Caspase Characterization**

Listing of MODULAR FOR, page 1 of 11/06/96

```

PROGRAM diff([input,output]);
(*n driver for routine DIFFER *)
uses int;
CONST
  over=0;
  under=100;
TYPE
  ftype = extended;
  glarray = ARRAY [1..over] OF ftype;
VAR
  i,j, count, index : integer;
  n1,n2,n3,n4,n5, Np, N, n, tried;
  overmax, undermax, distmax1,
  Compars, length,vert1, R4000;
  distmax,distmax2,distmax3,distmax4,
  distmax5,distmax6,distmax7,distmax8,
  distmax9,distmax10,distmax11,distmax12;
  wstart : glarray;
  w1 : ARRAY [1..2000] OF ftype;
  fmax : ARRAY [1..2000] OF ftype;
  p : ARRAY [1..over] OF ftype;
  q : ARRAY [1..over] OF ftype;
  dist;
  (* test *)
[=] MODULAR FOR =]
  0 TYPE
  1 char10 = string[10];
  2
  3 FUNCTION eng1(n:ftype):ftype;
  4 BEGIN
  5   eng1 := n
  6 END
  7
  8 PROCEDURE glopen(VAR info1:word; ftypemax:char10);
  9 BEGIN
  10  assign(info1,ftypemax);
  11  reset(info1);
  12 END
  13
  14 PROCEDURE deriv(n : ftype; p : glarray; dist:ftype);
  15   VAR dydx : glarray;
  16 (* The arrays p and dydx must carry the dimension given to
  17 them in the calling routine. *)
  18 BEGIN
  19   IF ( n < 0 OR ) then halt(0) else
  20     set := 0;
  21     compare := 0;
  22     distmax := dist;
  23     IF ((distmax < 0) OR (length > 100))
  24       then compare := 100;
  25     else compare := 0;
  26     compare := 0;
  27     compare := 0;

```


Listing of REXX/PL 840-include file REXX840.FAB, page 4

```

3      call
4      end
5      Parms{Xc1}:=21 (Source:=v1+ch(14(v1-v(2)))&
6      constant99(41constant9(1)-length(1,20))
7      v1:=v1+({out,loc(10+1) 0 0, ' ',v(1) 0 0, ' ', Source{Xc1} 0 0,
8      , v(2) 0 0, ' ', distance 0 0
9      , ' ',v(3)+v(2) 0 0, ' '
10     ,length(word(1,v)-space(1444+0 000% 0 0)) 0 0
11     , v(1) 0 0, ' ',v(2) 0 0)
12
13 2000
14 2000
15 20000
16
17     saveigs{Frac1, 'MCD840. gen'},
18
19     word1{Frac1},
20     v1{Frac1},
21     word1 :=(2000 0)
22     v1 := 0.000
23
24     distance:=v1{Frac1},
25     distance00:=0 0,
26     distance:=0 0 ,
27     distance:=0 0 ,
28     count:=0,
29     repeat
30     v1out1{count} := 0 0,
31     'out{count}',
32     word1 {word1{Frac1}},
33     v1out1{1}:=0 000,
34
35     Bp:=v(2) 0 0,
36     Ap:=0 (2000+0000 0)
37     B := (10 0) (space (1444+0 000% 0 0)
38     length:=2 000,
39     Width:=2 000
40     v1out1:=0 0,
41     v2 :=15 0000
42     count:=0 0
43     v1out1{v1out1:=out, v1, v2, v1out1},
44     {close output data file}
45     Flush{Frac1},
46     v1out1{Frac1}
47
48 2000

```

REFERENCES

- Ajmani, V. S. R. 1983. Mechanical Properties of the Core-Cell Under Quasi-static Radial Compression. *Trans. of the ASME* 105(4):1259-1268.
- Ajmani, V. S. R. and R. C. Chittenden. 1983. Poisson's Ratio and Elastic Modulus of Radially Compressed Fibrous Media - I. Shell Deformation Approximation. *Trans. of the ASME* 105(7): 523-529.
- Bagnall, G. B., C. E. Scherer, and D. E. Rabha. 1987. Harvesting and Handling of Fibers. In R. S. Noddy and R. S. Smith: Aquatic Plants for Waste Treatment and Resource Recovery. Magnolia Publishing, Inc., Orlando. pp. 589-609.
- Benedict, R. A. 1985. Fluid Mechanics. Class notes. University of Florida, Gainesville, FL.
- Bogert, D. R. 1949. The Effects of Aquatic Weeds on Flow in Overgrown Canals. *Proc. Soil Sci. of FL* 9:32-33.
- Chittenden, R. and V. S. R. Ajmani. 1984. Poisson's Ratio and Elastic Modulus of Radially Compressed Fibrous Media - II. Large Deformation Approximation. *Trans. of the ASME* 106(10):1262-1272.
- Class, C. R. and D. E. Frederick. 1979. Modeling and Analysis of Dynamic Systems. Douglas MacMillan Company, Boston.
- Colquhoun, R. R. and J. L. Doolittle. 1973. Mechanical Harvesting of Aquatic Plants. Tech. Rep. A-28-5. Rep. 1 Vol. 1. U.S. Army Engineer Waterways Exp. Station, Vicksburg, MS.
- Dabauis, T. A. and P. Stueber. 1984. Effects of Nitrogen and Ethyl Content on the Decomposition of the Water Hyacinth. *Hydrobiologia* 114(3):197-201.
- Gebhardt, P. R. and R. J. Cross. 1985. Fundamentals of Fluid Mechanics. Addison-Wesley Publishing Company, Reading.
- Gustafson, R. 1987. Correlation and Utilization of Flow Data on Flow Resistance and Heat Transfer for Cross Flow of Gases Over Tube Banks. *Trans. of the ASME* 109(7): 543-550.

Bernady, E. E. 1978. *Aeromagn. Spinning: Principles and Practice*. John Wiley and Sons. London.

Johansen, E. L. 1965. *Carpenter Bees*. Cambridge University Press. New York.

Robb, A. 1966. Response of Flowering Plants Under Natural Forces. In P. A. Salath. *Mechanical Harvesting of Aquatic Plants*. Rep. 2. 4-16-3 Utilization of Selected Handling Functions of Mechanical Harvest. (Appendix C). U. S. Army Engineer Research Dept. Station, Vicksburg MS. pp. 1-12.

Sage, E. R. and A. L. London. 1964. *Carpenter Bees*. Backlogers. McGraw Hill Book Company. New York.

Eller, C. P. and E. J. Ross. 1966. An Equation of Motion for Multiple Circular Beams in Free Fall in Confined Vertical Beams. *Trans. of ASME* 88(4): 468-473, 479.

Salping, G. B. Ross and E. T. Heller. 1971. Growth Characteristics, Field Potential, and Nutrient Content of Water Hyacinths. *Proc. of the Soil and Crop Sci. Soc. of Florida* (288) 20: 21-22.

Stanton, R. A. and E. Li. 1960. Elimination of Vegetation Channel Linkage. *J. Hydr. Div. ASCE*, 86(4): 1685-1693.

Stanton, R. A., E. Li, and D. Stacey. 1961. Plant Resistance in Vegetated Channels. *Trans. of the ASCE* 24(3): 564-58.

Stanton, R. A., E. Li and D. B. Stacey. 1966. A Stability Criteria for Vegetated Channels. *Trans. Am. Soc. Civ. Engr. Wash. D.C.* Lexington, KY. pp. 227-32.

Stanton, R. A. and T. E. Dray. 1972. Flooding Resistance in Open Channels. *J. Hydr. Div. ASCE*, 98(405): 712-28.

Stanton, R. A., T. E. Dray and R. H. Hill. 1969. Plant Resistance in Vegetated Channels. *J. of the Hyr. and Drain. Div. ASCE* 95(1027): 929-32.

Loeb, H. R. 1946. *Hydrodynamics*. Dover Publications. New York.

Lamarr, A. 1967. *Handbook of Hydraulic Engineering*. Ellis Horwood Limited. Chichester.

McIlroy, T. R. Chalm, E. Carter and E. Clifford Jr. 1976. Sequence and Pattern of Lateral Root Formation in Five Selected Species. *Ann. J. Bot.* 63(7): 800-809.

McQuiston, F. C. and G. D. Parker. 1968. *Boating, Yachting, and Air Conditioning: Analysis and Design*. John Wiley and Sons, Inc. New York.

Rehder, H. H. 1970. *Physical Properties of Plant and Animal Materials*. Gordon and Breach Science Publishers. New York.

- Panama, E. L. 1961. *Incompressible Flow*. John Wiley and Sons, Inc. New York.
- Parfitt, E. and T. Karim. 1961. Biology of the Water Hyacinth. *Soil Manage.* 14: 447-472.
- Petryk, S. and S. Brumfield III. 1973. Analysis of Flow Through Vegetation. *J. of the Hydr. Div. ASCE*. 99(877): 671-684.
- Parrell, E. E. 1978. Reed Growth - a Factor of Channel Roughness. In R. W. Burroughs: *Hydrology: Principles and Practice*. J. Wiley and Sons. London.
- Parrell, E. E. 1945. Flow in a Channel of definite roughness. *Trans. of ASCE* 110: 622-668.
- Potterki, L. and G. E. Tien-Jen. 1934. *Applied Hydraulics and Aeromechanics*. Dover Publications, Inc. New York.
- Pratt, R. E., E. P. Finnemore, R. A. Tebbelink, and R. T. Westering. 1966. *Statistical Design*. Press Syndicate of the University of Cambridge. New York.
- Raney, T., Ed. 1987. *Aquaphys*. 7(2): 11.
- Rana, W. E. 1968. Prediction and Transfer Coefficients for Single Particles and Packed Beds. *Chemical Engineering Progress* 68: 249-253.
- Raddy, A. F. 1966. Water Hyacinth Clogging Systems: I. Prediction. In R. E. Smith and J. E. Frank: *Methods from Hydraulics - a Systems Approach*. Elsevier Applied Science. London. pp. 127-138.
- Rao, R. G. and T. J. Palmer. 1960. Flow of Water in Channels Protected by Vegetative Linings. Technical Bulletin No. 307. U. S. Dept. of Agriculture. Washington, D. C. 105 pp.
- Rao, P. B. 2000. The Formation of Leaf-Matting in *Eichhornia crassipes*. *South Water Research*. Ind. New Res. J. 7:209-220.
- Reyns, R. B. and R. L. Aftersman. 1961. Roughness Spacing in Field Open channels. *J. of the Hydr. Div. ASCE*. 87(974): 124-128.
- Richards, J. D. 1966. The 1966 Florida Aquatic Plant Survey. Bureau of Aquatic Plant Management. Tallahassee.
- Schlichting, H. 1979. *Boundary-Layer Theory*. McGraw Hill Book Company. New York.
- Shaw, J. L., A. T. Murphy and R. E. Richardson. 1971. *Introduction to Aquatic Dynamics*. Addison-Wesley Publishing Company. Reading.
- Stall, R. P. and C. E. Rahn. 1962. Seasonal Variations of Manning's Roughness Coefficients in a Subtropical River. *Trans. of the ASCE* 32(7): 112-119.

- Hilbel, G. 1988. *Handbook of Agricultural Statistics*. Elsevier, New York.
- Jean, R. and J. H. Terris. 1980. *Principles and Procedures of Statistics*. McGraw-Hill Book Company, Inc. New York.
- Stephens, R. L., E. D. Blackburn, B. J. Samsen, and L. W. Nelson. 1983. Flow Relationships by Channel Width and Their Control. *J. of the Irr. and Drain. Div. ASCE* 109(180) 34-42.
- Strickler, A. 1959. Beiträge zur Frage der Geschwindigkeit Formel und der Durchlässigkeit-inkten für Ström, Rausch und Geschlossene Leitungen. Mitteilungen des Eidgenössischen Anses für Wasserwirtschaft No. 15g. Bern, Switzerland. Schweizer, F. M. and T. E. Rantz. 1981. *Stream Test Methods for Geomorphics*. Van Nostrand Reinhold Company, New York.
- Tarakan, L. S. and V. S. Gureva. 1981. The Drag Coefficients of Elastic Spheres Moving in Steady and Accelerated Motion in a Turbulent Field. *American Institute of Chemical Eng.* 7:925-945.
- Tucker, C. E. 1981. Relationships Between Oxygen Sensing and the Composition of Three Floating Aquatic Macrophytes. *Hydrobiologia* 80 72-80.
- Turner, G. R. and B. Gosselink. 1984. Shallow Flow of Water Through Submerged Vegetation. *Agricultural Water Management* 8(4) 275.
- Waters, C.F., G. Chalmers and B. Landon. 1986. Morpho-anatomical Studies of *Elodea canadensis* (Lour.) Solms Water Species. *Geographic Distribution Sci. Appl. Sci. Bull.* 20:1-11.

BIOGRAPHICAL SKETCH

Raposa Jose Petrelli was born in Grand Forks, North Dakota. She moved while still in primary school to Evelyns, Minnesota. There, she graduated from high school in 1953. In the University of Minnesota (Duluth), she received in 1955 a B.S. degree in Biology. The Pennsylvania State University awarded her a M.S. degree in agricultural engineering in 1957.

Before she started graduate studies at the University of Florida, she worked for several years in Mexico. There, she worked on projects in the areas of physical properties, project engineering and development, and food engineering. She became interested in aquatic plant culture as an alternative to conventional sewage treatment. It was that interest that brought her to the University of Florida for graduate studies.

I certify that I have read this study and that in my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy.


Wayne Edwin Thomas
Professor of Agricultural Engineering

I certify that I have read this study and that in my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy.


Larry W. Russell, Chairman
Professor of Agricultural Engineering

I certify that I have read this study and that in my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy.


Alan H. Swearing
Associate Professor of Agricultural Engineering

I certify that I have read this study and that in my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy.


Del Wheeler
Professor of Agricultural Engineering

I certify that I have read this study and that in my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy.


Chang T. Liu
Professor of Engineering Science

This dissertation was submitted to the Graduate Faculty of the College of Engineering and to the Graduate School and was approved as partial fulfillment of the requirements for the degree of Doctor of Philosophy

December 1966

Richard D. Smith
Name: College of Engineering

Name: Graduate School